



DEPARTMENT of PUBLIC WORKS and PLANNING
DEVELOPMENT SERVICES DIVISION



Approximate Location of
Project Site



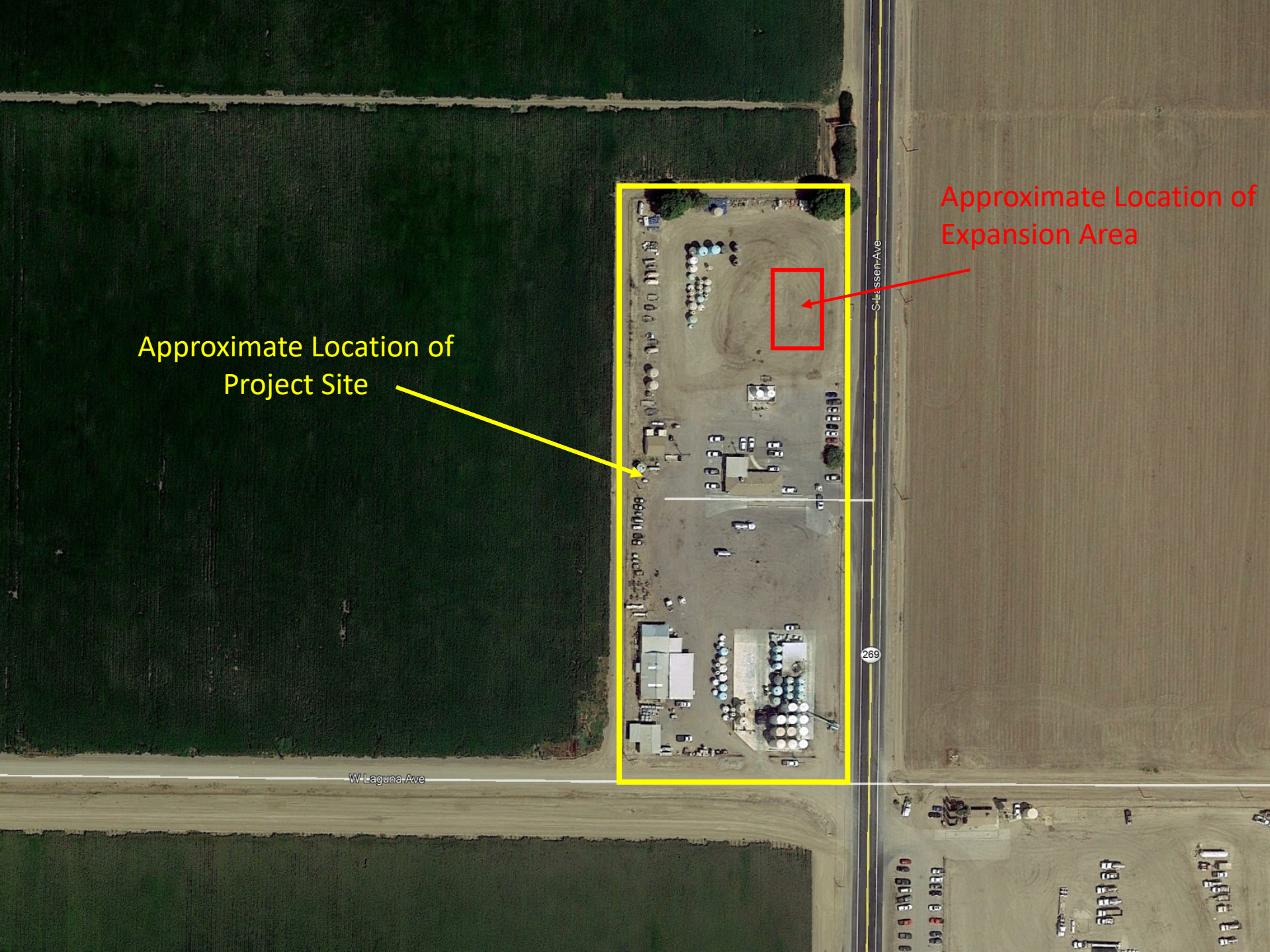
W Laguna Ave

Silkasser Ave

269

Approximate Location of Project Site

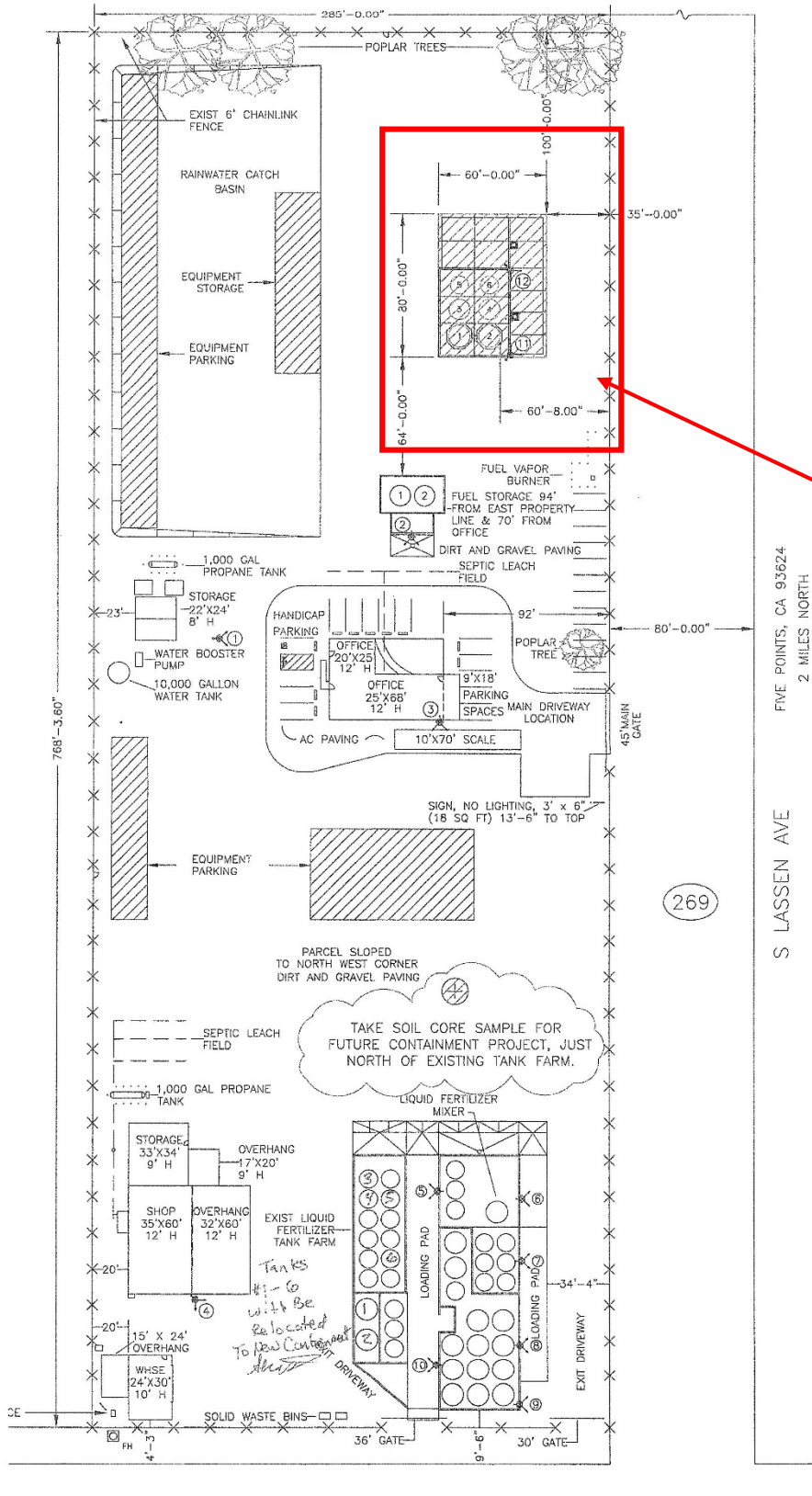
Approximate Location of Expansion Area



W Laguna Ave

Silkasser Ave

269



FIVE POINTS, CA 93624
2 MILES NORTH

S LASSEN AVE

269

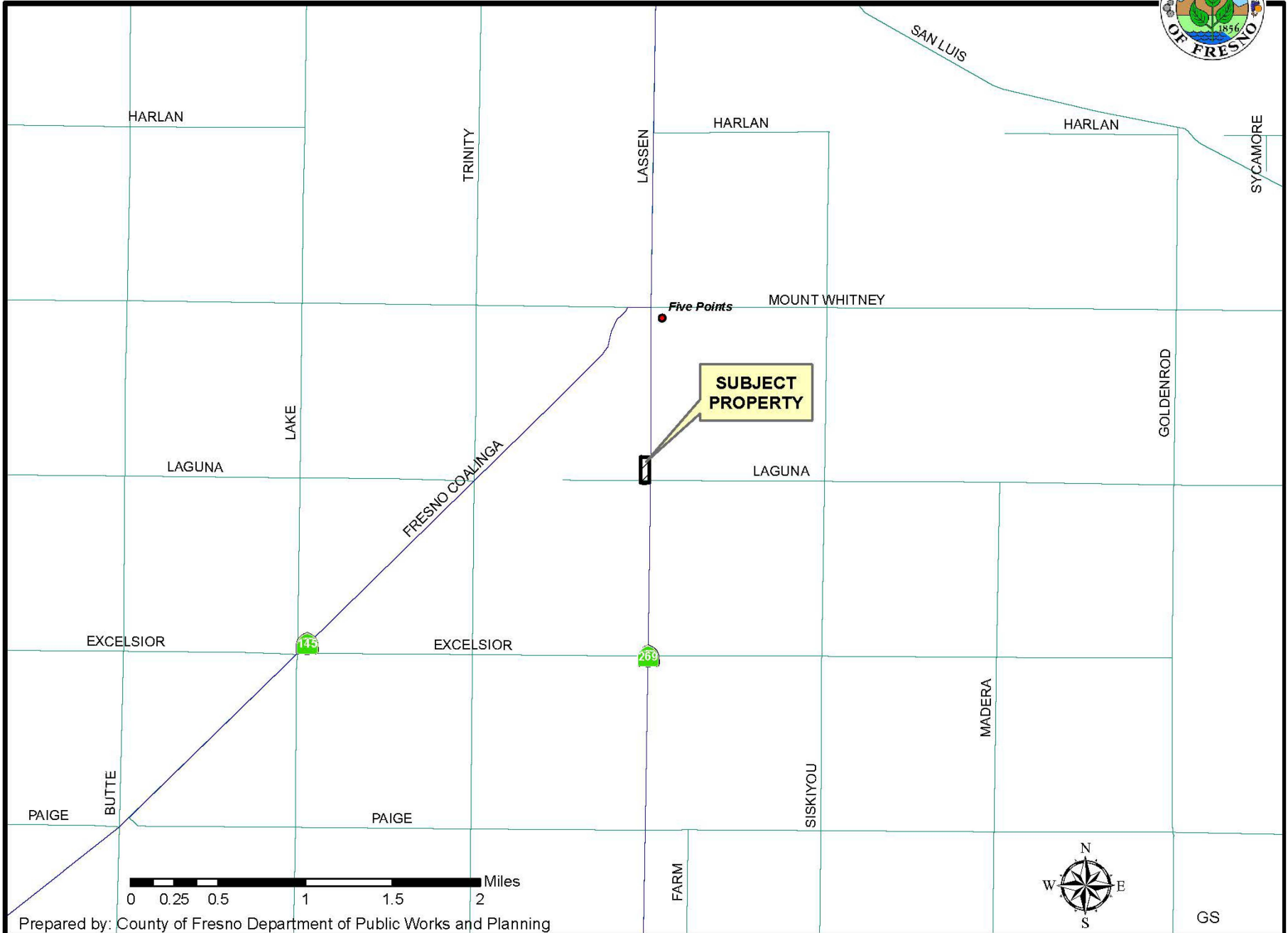
Proposed Expansion Area

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GENERAL SERVICES DIVISION

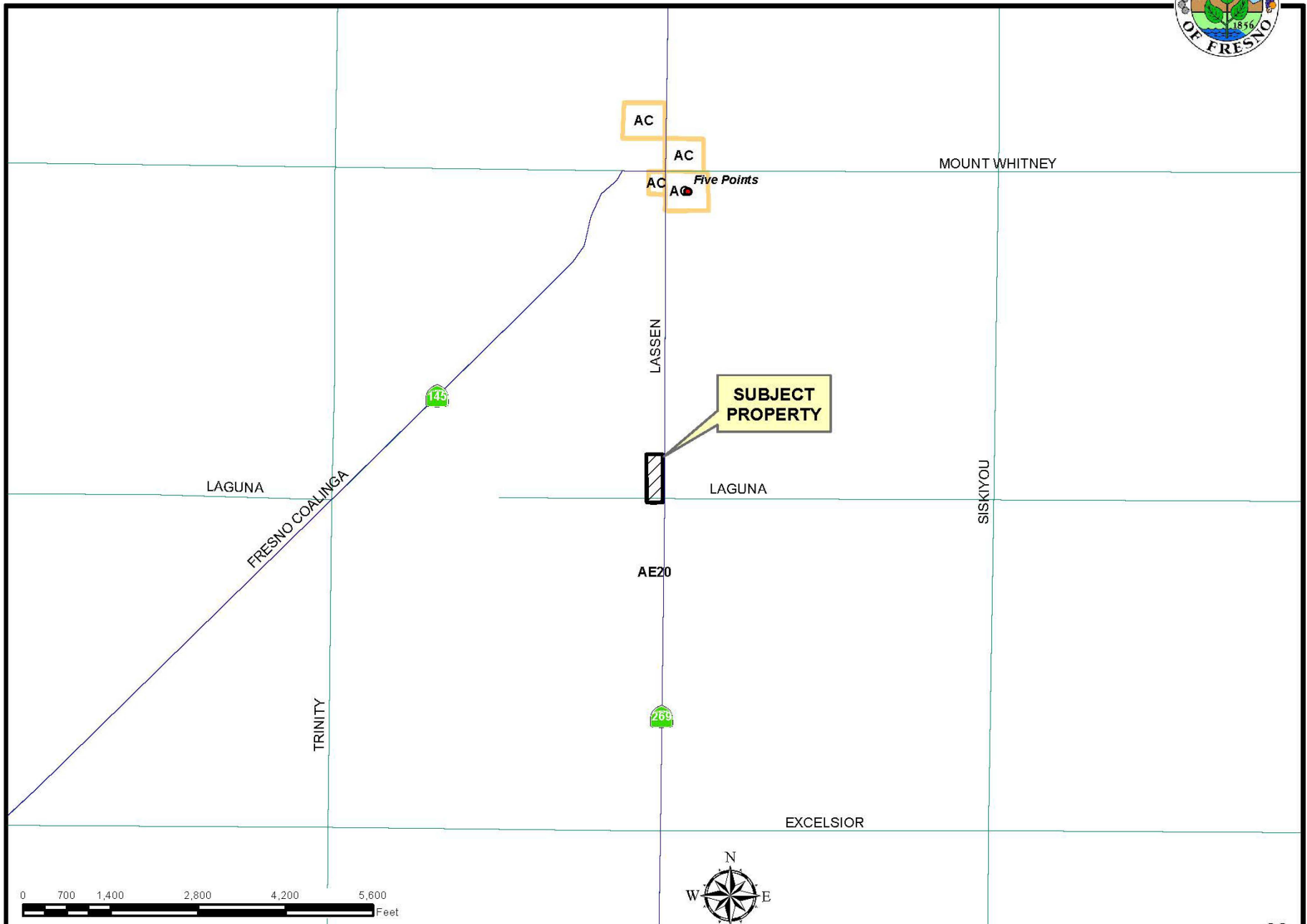
Tanks #1-6 will be relocated to new center area

TAKE SOIL CORE SAMPLE FOR FUTURE CONTAINMENT PROJECT, JUST NORTH OF EXISTING TANK FARM.

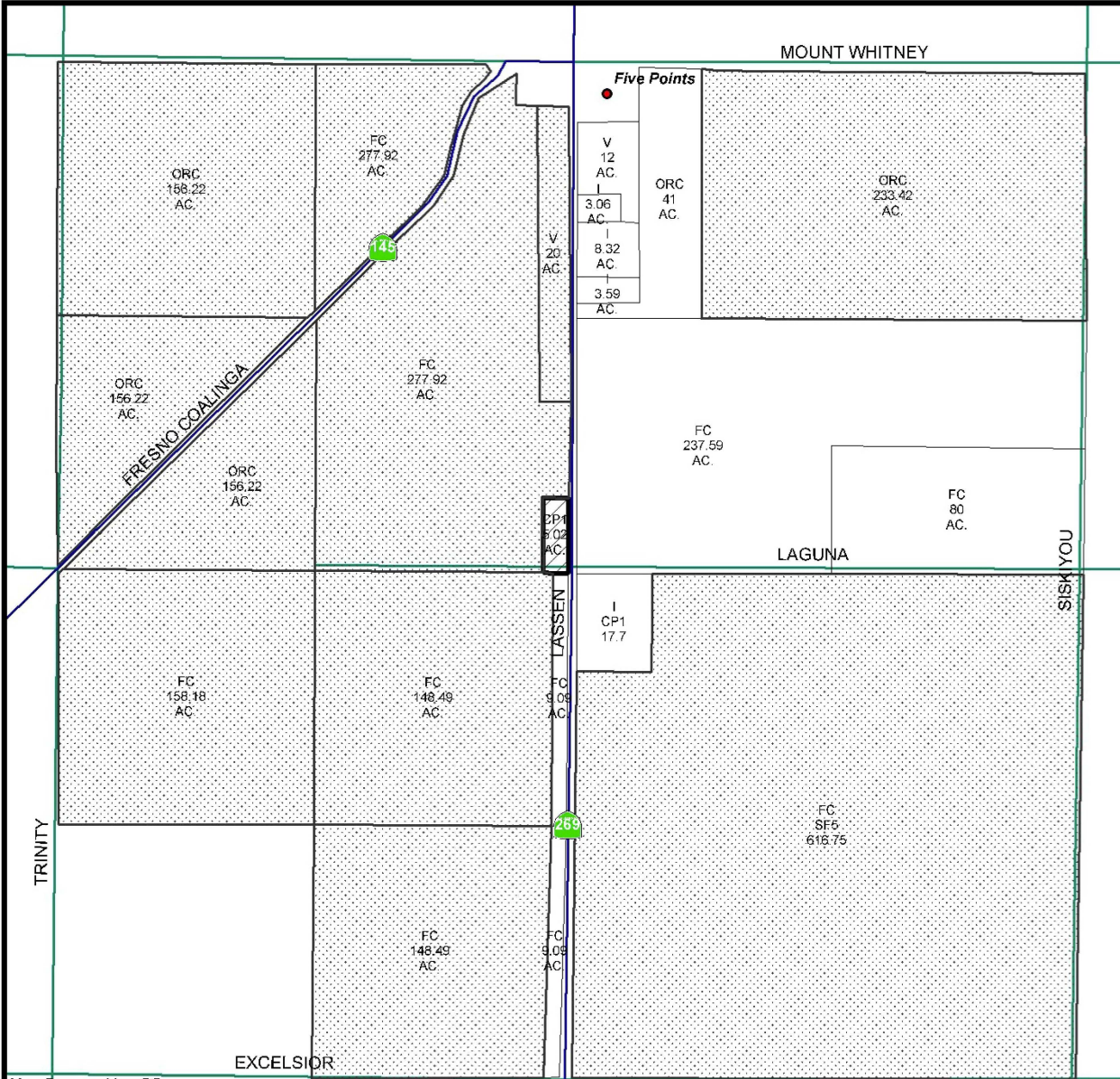
LOCATION MAP



EXISTING ZONING MAP



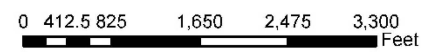
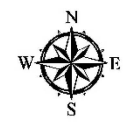
EXISTING LAND USE MAP



LEGEND	
CP#	OFFICE COMM./PROF
FC	FIELD CROP
I	INDUSTRIAL
ORC	ORCHARD
SF#	SINGLE FAMILY RESIDENCE
V	VACANT

LEGEND:

- Subject Property
- Ag Contract Land



Five Findings for a Conditional Use Permit

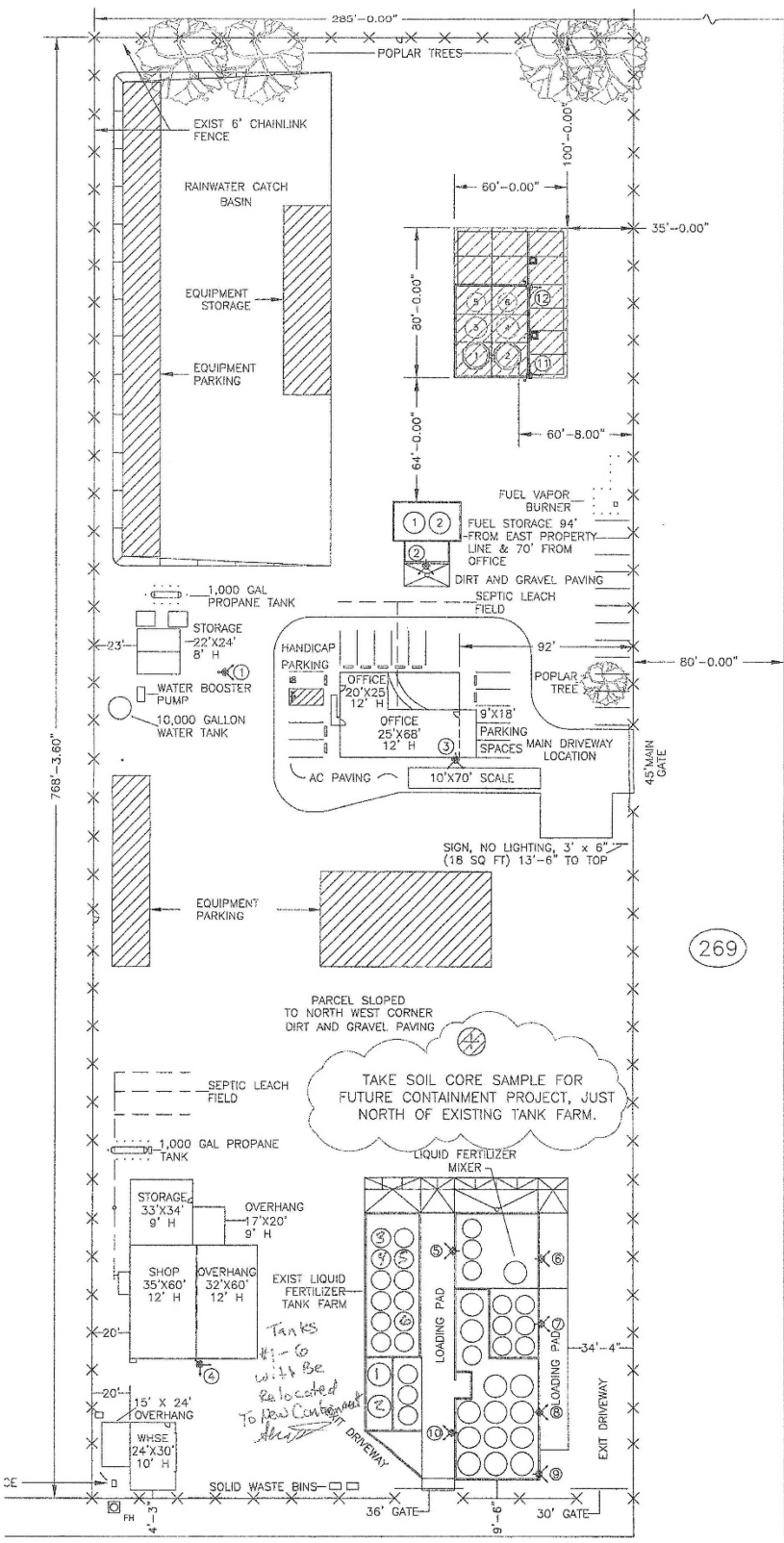
1. The site for the proposed use is adequate in size and shape to accommodate said use and all yards, spaces, walls, and fences, parking, loading, landscaping, and other features required by this Division, to adjust said use with land and uses in the neighborhood.
2. The site for the proposed use relates to streets and highways adequate in width and pavement type to carry the quantity and kind of traffic generated by the proposed use.
3. The proposed use will have no adverse impact on abutting property and surrounding neighborhood or permitted use.
4. The proposed development is consistent with the General Plan.
5. That the conditions stated in the resolution are deemed necessary to protect the public health, safety, and general welfare





DEPARTMENT of PUBLIC WORKS and PLANNING DEVELOPMENT SERVICES DIVISION





FIVE POINTS, CA 93624
2 MILES NORTH

S LASSEN AVE

(269)

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JAN 23 2020
DEPARTMENT OF PUBLIC WORKS
CONSTRUCTION AND PLANNING
DESIGN DIVISION, TECHNICAL SERVICES

1.0 Site Preparation
Based on Geotechnical Report by BSK ASSOCIATES, Date 12/27/18,
Project Number G18-318-10F

1.1 It is recommended that the containment structure be over-excavated to at least 12 inches below preconstruction site grades to a minimum depth of 2 feet below proposed foundations, to at least 12 inches below existing improvements to be removed, or to the depth required to remove any fill soils, whichever is greater. The zone of over-excavation should include the entire footprint of the containment structure and a minimum of five (5) feet beyond the containment structure. Upon approval of the bottom of the over-excavation by a representative of BSK Associates, the exposed bottom should be classified to a depth of 8 inches, moisture conditioned to 45 relative optimum moisture content and compacted to a minimum of 90 percent relative compaction prior to fill placement.

1.2 It is recommended that the concrete drive area planned adjacent to the containment structure be over-excavated to at least 12 inches below preconstruction site grades, to at least 12 inches below improvements to be removed, 12 inches below the bottom of the proposed aggregate base, or to the depth required to remove fill soil (if any), whichever is greater. The zone of over-excavation should include the entire footprint of the proposed drive area and a minimum of three (3) feet beyond the drive area. The upper 12 inches of subgrade below the aggregate base for the drive area should be compacted to at least 95 percent relative compaction. Upon approval of the bottom of the over-excavation by a representative of BSK Associates, the exposed bottom should be classified to a depth of 8 inches, moisture conditioned to slightly above 45 moisture content and compacted to a minimum of 90 percent relative compaction prior to fill placement.

1.3 It is recommended that extra care be taken by the contractor to ensure that the horizontal and vertical extent of the over-excavation and compaction extends to the site preparation recommendations presented in this report. BSK Associates is not responsible for measuring and verifying the horizontal and vertical extent of over-excavation and compaction. The contractor should verify in writing to the owner and BSK Associates that the horizontal and vertical over-excavation limits were completed in accordance with the recommendations of the report, the project plans, and the specifications (that most stringent apply).

It is recommended that this verification be performed by a licensed surveyor. The verification should be provided prior to requesting pad certification from BSK Associates or accepting for foundations.

1.4 If soil is unstable soils are encountered during excavation or compaction operations, BSK Associates should be notified as these soil conditions are observed and additional recommendations presented, as considered appropriate.

1.5 Final grading shall provide a 2% cross slope on grade with a smooth, finished and material in place. Both the finished pad (before fill placed) and the aggregate base shall be proof rolled and shall not depress more than one-half (1/2) inch under the area of a fully loaded commercial truck or equipment. If depressures more than one-half (1/2) inch occur, the contractor shall perform remedial grading to correct this condition.

1.6 The containment structure and concrete drive area should be underlain by a minimum of 3 inches of California Class 2 aggregate base. The aggregate base shall be compacted to a dry density of at least 90 percent of the maximum dry density as determined by ASTM Test Method D1557.

1.7 All fill required to bring the site to final grade should be placed as engineered fill. In addition, all entire site over-excavation should be compacted as engineered fill.

1.8 The contractor is responsible for the disposal of concrete, asphaltic concrete, rock spalls, etc. that must be exported from the site. Individuals, facilities, agencies, etc. may require analytical testing and other assessments of these materials to determine if these materials are acceptable for the intended use by the receiving party. This Contractor is responsible to perform the tests, assessments, etc. necessary to determine the appropriate method of disposal. In addition, the Contractor is responsible for the cost to dispose of these materials in a legal manner.

2.0 Engineered Fill

2.1 Site sand, Expansion Index (EI) <10 will be suitable for use as fill material below the recommended aggregate base to support the structure loads, provided they are free of organic (less than 3 percent by weight), roots, debris, and particles larger than 1/2 inches in any dimension. Material conditioned to slightly above optimum, and compacted to 95% relative compaction. The existing ground material may be used under the area of a fully loaded commercial truck or equipment. If depressures more than one-half (1/2) inch occur, the contractor shall perform remedial grading to correct this condition.

2.2 The compressibility of the native soil is dependent upon the moisture content, subgrade conditions, degree of mixing, type of equipment, as well as other factors. The evaluation of such factors was beyond the scope of this report; therefore, we recommend that they be evaluated by the Contractor, during preparation of bids and construction of the project.

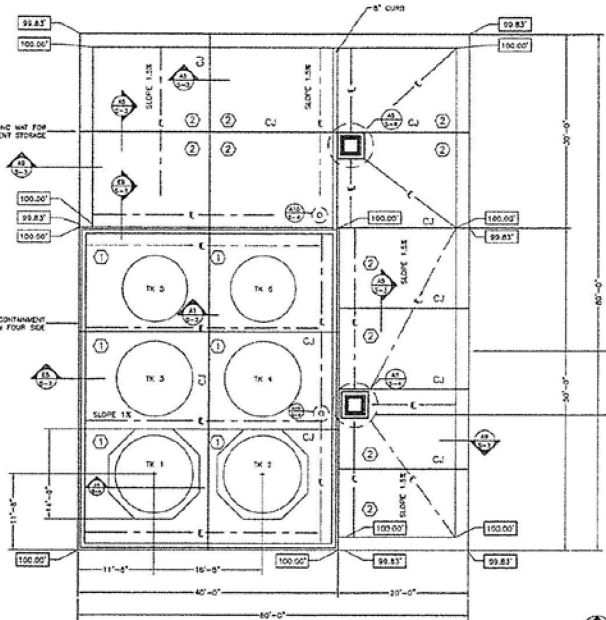
2.3 Imported fill soil, if required for the project, should be non-compressible, non-expansive and granular in nature and contain enough fine grained material (blades) to slice cutting "test" loading trenches with the following acceptance criteria recommendations:

Prior to being transported to the site, the imported material shall be certified by the Contractor and the supplier (to the satisfaction of the Owner and BSK Associates) that the soils do not contain any environmental contaminants regulated by local, state or federal agencies have jurisdiction. In addition, BSK Associates should be requested to sample and test the material to determine compliance with the above geotechnical criteria. Contractors should provide a minimum of 7 working days to complete the testing.

2.4 Native and imported engineered fill soil should be placed in lifts (16" approximately) 6 inches thick or less, moisture-conditioned to slightly above optimum moisture content, and compacted to a dry density of at least 90 percent of the maximum dry density as determined by ASTM Test Method D1557. The upper 12 inches of engineered fill below aggregate base in the area of the proposed concrete treatments should be compacted to a dry density of at least 90 percent of the maximum dry density as determined by ASTM Test Method D1557. Additional fill should be placed if the previous lift did not meet the required dry density or if soil conditions are not stable.

(continued on DWG S-2)

NO	STATUS	TYPE	DN DIMENHS	HEIGHT INCHES	MATERIAL STORED	VOLUME GALLONS	LIQ UNIT WT LB/GAL	EQ TOTAL WT LBS	TANK DEAD WT LBS	TOTAL TANK WT LBS	PART NO
1	NEW	PC CONE	144"	228"	KAPAM	15,943	11.7	186,767	5,000	191,767	-
2	NEW	PC CONE	144"	228"	10FAM	15,943	11.7	186,767	5,000	191,767	-
3	NEW	POLY	144"	243"	OE	15,500	7.11	110,200	4,000	114,200	-
4	NEW	POLY	144"	243"	OE	15,500	7.11	110,200	4,000	114,200	-
5	NEW	POLY	320"	149"	OE	6,500	7.11	46,215	2,500	48,715	-
6	NEW	POLY	320"	149"	OE	6,500	7.11	46,215	2,500	48,715	-



①	15" THICK MIN CONCRETE MAT FOUNDATION W/ #5 @ 9" OC EW @ BTM & #4 @ 12" OC EW @ TOP.
②	9" THICK CONCRETE MAT FOUNDATION W/ #4 @ 12" OC EW CTRD IN MAT.
CJ	CONTROL JOINT - SEE A1 & A5/S-3

- NOTE:
- COORDINATE FINISHED ELEV AND STRUCTURE LOCATION WITH OWNER BEFORE CONSTRUCTION.
 - SEE DWG S-4 FOR TANK PEDESTAL LOCATION AND ELEV.

DESCRIPTION OF VERIFICATION AND INSPECTION WORK	INSPECTION FREQUENCY
1. VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	- PERIODIC
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	- PERIODIC
3. PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.	- PERIODIC
4. VERIFY USE OF PROPER MATERIALS, GEOMETRIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.	- CONTINUOUS
5. PRIOR TO PLACEMENT OF CONTROLLED FILL OBSERVE SUBGRADE AND VERIFY THAT THE SITE HAS BEEN PREPARED PROPERLY.	- PERIODIC

DESCRIPTION OF VERIFICATION AND INSPECTION WORK	INSPECTION FREQUENCY	REFERENCE STANDARD
1. INSPECTION OF REINFORCING STEEL AND PLACEMENT.	PERIODIC	ACI 318, CH 20, 25.2, 25.7, 26.8.1-26.8.3
2. INSPECT SOILS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE.	PERIODIC	ACI 318, CH 17.5.2
3. VERIFYING USE OF APPROVED DESIGN MIX.	PERIODIC	ACI 318, CH 19, 28.4.3, 28.4.4
4. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE AIR TEMPERATURE OF THE CONCRETE.	CONTINUOUS	ASTM C 173, ASTM C 91, ACI 318, CH 26.4.26.17
5. INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	CONTINUOUS	ACI 318, CH 26.3.3, 26.5.5

BASIS OF DESIGN: CBC 2016 OR PER LOCAL JURISDICTION AND AMENDMENTS

- DESIGN LOADS:
 - UNF LOADS
 - ROOF LOADS.....NA
 - DEAD LOAD.....WEIGHT OF TANKS
 - CONCENTRATED LOADS.....NONE
 - WIND LOAD:
 - (1) CATEGORY = I
 - (2) EXPOSURE CATEGORY =.....C
 - (3) WIND SPEED =.....110 MPH
- DESIGN LOAD:
 - RISK CATEGORY = I (CBC TABLE 1604.5)
 - S_s = 1.175
 - S₁ = 0.305
 - S₀ = 0.807
 - S₁ = 0.450
 - SOIL CLASS = F
 - SOIL STRENGTH-CAPACITY-RESISTING SYSTEM
 - FLAT BOTTOM TANKS

RECEIVED
COUNTY OF FRESNO
JAN 23 2020

Drawings Prepared by
S & R SPECIALTY EQUIPMENT CO INC
P.O. BOX 505 - 830 JEPSEN AVE
CORCORAN, CA 93212
(559) 992-4191 FAX (559) 992-8341

Working Title: NUTREN AG SOLUTIONS, FIVE POINTS, CA CONTAINMENT STRUCTURE AND TANK FOUNDATION

Designed	Project No	1002
Drawn	Date	1/6/20
Checked	Drawing No	
Approved		
Date		2/16/20

S-1

DEPARTMENT OF PUBLIC WORKS
AND PLANNING
DEVELOPMENT SERVICES DIVISION















