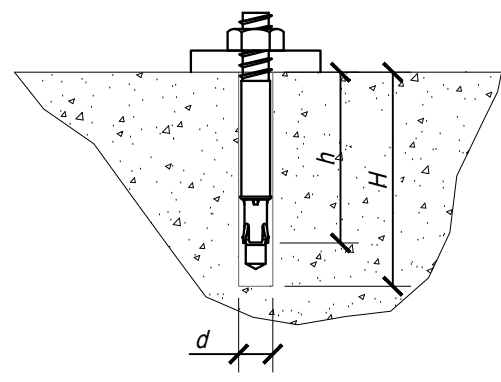


9. POST-INSTALLED WEDGE-TYPE ANCHORS IN CONCRETE AND MASONRY

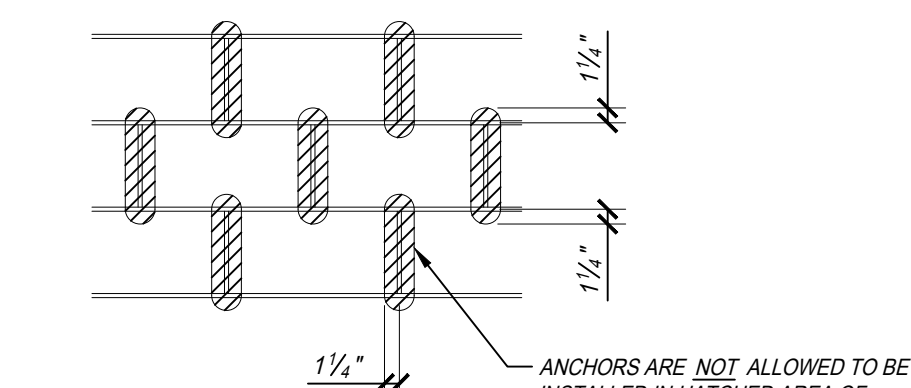
ANCHOR DIAMETER (in.)	POST-INSTALLED WEDGE-TYPE ANCHORS		
	INSTALLATION TORQUE (#.8)		HILTI KB-3 ICC ESR-1385
	HILTI KB-TZ ICC ESR-1917	SIMPSON STRONG-BOLT ICC ESR-1771	MASONRY
1/4"	- NOT USED -	- NOT USED -	4
3/8"	25	- NOT USED -	15
1/2"	40	50	25
5/8"	60	85	65
3/4"	110	160	120



h = EFFECTIVE DEPTH OF ANCHORS AS INDICATED ON DRAWINGS
H = DEPTH OF DRILLED HOLE PER ICC REPORT
D = HOLE DIAMETER = ANCHOR DIAMETER

NOTES

- TORQUE TEST PROCEDURES ARE FOR HILTI KB-TZ WEDGE-ANCHORS INSTALLED IN ACCORDANCE WITH ICC ESR-1917 AND SIMPSON STRONG-BOLT WEDGE-ANCHORS INSTALLED IN ACCORDANCE WITH ICC ESR-1771 AND HILTI KB-3 WEDGE-ANCHORS INSTALLED IN ACCORDANCE WITH ICC ESR-1385.
- ALL ANCHORS SHALL BE TORQUE TESTED. IOR OBSERVATION OF INSTALLATION TORQUE SHALL BE CONSIDERED ACCEPTABLE TESTING.
- THE TORQUE TESTING OF WEDGE ANCHORS SHALL BE DONE IN THE PRESENCE OF THE PROJECT INSPECTOR (ICR) OR A SPECIAL INSPECTOR* MAY PERFORM THIS TEST. A REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO THE ENFORCEMENT AGENCY AND THE ARCHITECT. IF ANY ANCHOR FAILS THE TESTING REQUIREMENTS THAT ANCHOR SHALL BE REPLACED BY ANOTHER ANCHOR PER ENGINEER'S DIRECTIVE.
- ANCHOR DIAMETER REFERS TO THE THREAD SIZE.
- TEST EQUIPMENT (INCLUDING TORQUE WRENCHES) IS TO BE CALIBRATED BY AN APPROVED TESTING LABORATORY IN ACCORDANCE WITH STANDARD RECOGNIZED PROCEDURES.



ANCHOR INSTALLATION IN CMU WALL

8. ADDITIONAL SPECIAL INSPECTION ITEMS

TASK	SOILS PER CBC TABLE 1705A.6	
	CONTINUOUS	PERIODIC
VERIFY MATERIAL BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY		X
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL		X
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS		X
VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	X	
PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUB-GRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY		X

TASK	CONCRETE PER CBC TABLE 1705A.3	
	CONTINUOUS	PERIODIC
1. INSPECT PLACEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY LIFTS		X
2. REINFORCING BAR WELDING:		X
A) VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706		X
B) INSPECT SINGLE-PASS FILET WELDS, MAXIMUM 5/16"	X	
C) INSPECT ALL OTHER WELDS	X	
3. INSPECT ANCHORS CAST IN CONCRETE		X
4. INSPECT ANCHORS POST INSTALLED IN HARDENED CONCRETE MEMBERS		X
A) ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	X	
B) MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.A		X
5. VERIFY USE OF REQUIRED DESIGN MIX		X
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X	
7. INSPECT CONCRETE AND SHOROTAKE PLACEMENT FOR PROPER APPLICATIONS	X	
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		X
9. INSPECT PRESTRESSED CONCRETE FOR:		X
A) APPLICATION OF PRESTRESSING FORCES	X	
B) GROUTING OF BONDED PRESTRESSING TENDONS	X	
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS		X
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS		X
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED		X

6. LIGHT-GAUGE STEEL FRAMING

- DESIGN OF LIGHT-GAUGE STEEL HAS BEEN BASED ON THE 2016 CBC, CHAPTER 2211A. ALL WORK SHALL CONFORM TO THE CALIFORNIA BUILDING CODE AND THE AISI N95.
- ALL DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND ALL OTHER CONTRACT DRAWINGS AND SPECIFICATIONS.
- DETAILS SHOWN ON STRUCTURAL DRAWINGS ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS.
- DIMENSIONS SHOWN SHALL TAKE PRECEDENCE OVER SCALE ON PLANS, SECTIONS, AND DETAILS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR ENGINEER IN RESPONSIBLE CHARGE OF THE PROJECT IMMEDIATELY.
- CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FLOOR OR ROOF FRAMING MEMBERS. LOAD SHALL NOT EXCEED DESIGN LIVE LOAD.
- ALL STUD JOIST AND MISCELLANEOUS MATERIAL SHALL HAVE STIFFENED FLANGES WITH 90° RETURNS AND SHALL BE MANUFACTURED IN ACCORDANCE WITH THE LATEST AISI SPECIFICATION. MATERIAL SHALL CONFORM TO THE FOLLOWING:
MATERIAL STRENGTH:
16 GAUGE AND HEAVIER - 50 KSI MIN. YIELD
18 GAUGE AND LIGHTER - 33 KSI MIN. YIELD
MATERIAL DESIGN THICKNESS:
12 GA = .0171"
14 GA = .0171"
16 GA = .0168"
FASTENERS: ASTM C115 (SCREWS SHALL EXTEND THRU STEEL CONNECTION A MIN. OF 3 EXPOSED THREADS)

- METAL-TO-METAL: SELF-TAPPING SHEET METAL SCREWS
- | METAL-TO-METAL SCREW SIZE | NOMINAL DIAMETER |
|---------------------------|------------------|
| #8 | 1/16" |
| #10 | 1/8" |
| #12 | 3/16" |
- WOOD-TO-METAL: SELF-TAPPING SHEET METAL SCREW WITH BUGLE HEAD DESIGNED FOR ATTACHMENT OF WOOD TO METAL
- | WOOD-TO-METAL SCREW SIZE | NOMINAL DIAMETER |
|--------------------------|------------------|
| #8 | 1/16" |
| #10 | 1/8" |
| #12 | 3/16" |

- METAL-TO-WOOD: WOOD SCREW WITH FLUSH BEARING OR HEAD TO METAL SURFACE. PRE-DRILL METAL SURFACE WITH HOLES EQUAL TO SCREW DIAMETER
- | METAL-TO-WOOD SCREW SIZE | NOMINAL DIAMETER |
|--------------------------|------------------|
| #8 | 1/16" |
| #10 | 1/8" |
| #12 | 3/16" |

- WIRE TYING OF FRAMING IS NOT ALLOWED.
- ALL COMPONENTS SHALL BE CUT SQUARELY OR AS REQUIRED FOR AN ANGULAR FIT TO RECEIVING MEMBERS. BENT, DISTORTED OR OTHERWISE DAMAGED COMPONENTS SHALL NOT BE USED.
- ALL JOIST MEMBERS SHALL BE UN-PUNCHED UNLESS OTHERWISE INDICATED. BEARING AND NON-BEARING PARTITION WALL STUDS MAY BE PUNCHED.
- LATERAL STRAPBRACING FOR WALL STUDS SHALL BE APPLIED TO BOTH SIDES OF WALL AT 8'-0" VERTICAL SPACING FOR WALLS EXCEEDING 8'-0" IN HEIGHT AND INDICATED OTHERWISE ON THE DRAWINGS. INSTALL STRAPBRACING @ 24"oc VERT. SPACING WHERE WALL FINISH DOES NOT OCCUR. (SEE DETAIL 0551)
- AXIALLY LOADED STUDS SHALL HAVE FULL BEARING AGAINST INSIDE TRACK WEB PRIOR TO STUD AND TRACK ATTACHMENT. SPLICES IN STUDS SHALL BE PERMITTED.
- STUD WALLS SHALL BE BOLTED TO THE SLAB WITH ANCHOR BOLTS AS DETAILED. SHOT PINS AND EXPANSION ANCHORS ARE NOT ALLOWED AT CURBS. SHOT PINS ARE NOT ALLOWED AT SLAB EDGES. INTERIOR WALLS MAY BE ATTACHED WITH 1/2" DIAMETER x 1/2" POWERDRIVEN FASTENERS AT 24"oc UNLESS NOTED OTHERWISE.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING TEMPORARY BRACING OF WALLS, DURING ERECTION.
- BOTTOM SILL TRACK AT CURVED WALLS (IF THEY OCCUR ON THIS PROJECT) SHALL BE COLD FORMED TO REQUIRED RADIUS, CUTTING OFF FLANGE AND WEB SECTIONS IS NOT ALLOWED FOR PURPOSES OF BENDING SCHEDULE TO REQUIRED RADIUS.
- MANUFACTURER SHALL BE A MEMBER OF THE SSMA - STEEL STUD MANUFACTURERS ASSOCIATION. SECTIONS OF METAL COMPONENTS SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

DEPTH	FLANGE WIDTH	TRACK
600	162	32

TYPE	MARK	MIN. EFFECTIVE PROPERTIES	ASTM SPEC	F _y (ksi)	F _t (ksi)	AREA (in ²)	L _p (in)
STRUCTURAL (S) STUD SECTION PROPERTIES							
400 SF	A005162-33	0.692	A955	33	0.278	0.500	
400 SF	A005162-43	0.862	A463	33	0.357	0.500	
600 SF	B005162-33	1.793	A955	33	0.344	0.500	
600 SF	B005162-43	2.376	A955	33	0.447	0.500	
600 SF	B005162-54	2.861	A955	50	0.560	0.500	
STRUCTURAL (T) TRACK SECTION PROPERTIES							
400 TT	A007150-33	0.519	A955	33	0.242	N/A	
400 TT	A007150-43	0.719	A955	33	0.316	N/A	
600 TT	B007150-33	1.861	A955	33	0.371	N/A	
600 TT	B007150-43	1.868	A955	33	0.405	N/A	
600 TT	B007150-54	2.671	A955	50	0.509	N/A	

THICKNESS GAUGE (MILS)
FLANGE WIDTH (162/100 = 1.625)
SECTION TYPE (STUD = S OR TRACK = T)
DEPTH (600 = 600/100 = 6)
*L_p LENGTH IS BASED ON FLANGE WIDTH

- CHANNELS, ANGLES & BASE PLATES - ASTM A36, Gr. A
- STRUCTURAL PIPE - ASTM A53, GRADE B
- STRUCTURAL HSS RECTANGULAR TUBING - ASTM A500, GRADE C (F_y = 50 KSI)
- MISC. METALS - ASTM A36, Gr. A
- STANDARD BOLTS - ASTM A307, Gr. A, TYPICAL UNLESS NOTED OTHERWISE.
- STANDARD AND HEAVY-HEX NUTS - ASTM A563 - TYPICAL UNLESS NOTED OTHERWISE.
- STANDARD ANCHOR BOLTS - ASTM F1554 (Gr. 36 OR Gr. 55 WHERE NOTED)
- WASHERS - AS REQUIRED BY THE AISI, ROSS, SECTION 6 - USE OF WASHERS
- WELDING ROD - HEAVILY COATED, CONFORMING WITH A.I.S. SPECIFICATIONS FOR ARC WELDING; ELECTRODES OF CLASSIFICATION NUMBERS SUITABLE FOR THE WORK TO BE DONE.

7. SPECIAL INSPECTIONS

- A STATEMENT FOR SPECIAL INSPECTION PREPARED BY THE SPECIAL INSPECTION AGENCY OF RECORD IN ACCORDANCE WITH 2016 CBC 1703A.1 MUST BE SUBMITTED PRIOR TO ISSUANCE OF PERMITS. THE SPECIAL INSPECTION AGENCY MUST BE CERTIFIED BY THE ICC (INTERNATIONAL CODE COUNCIL) AND APPROVED BY THE BUILDING OFFICIAL. THE PROPOSAL MUST INDICATE THAT SPECIAL INSPECTION HAS BEEN OBTAINED BY THE OWNER OR THE OWNER'S AGENT, BUT NOT THE CONTRACTOR OR THE PERSON RESPONSIBLE FOR THE WORK. THE PROPOSAL MUST IDENTIFY THE SCOPE OF REQUIRED INSPECTIONS, LIST THE INDIVIDUALS PERFORMING THE INSPECTIONS INCLUDE CURRENT INDIVIDUAL CERTIFICATIONS AS WELL AS THE LABORATORY'S CERTIFICATION, AND MUST BE ATTACHED TO EACH SET OF PLANS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR SCHEDULING ALL SPECIFIED INSPECTIONS AND TESTING WITH THE INSPECTOR/TESTING AGENCY. SEE SPECIFICATIONS FOR REQUIRED INSPECTIONS AND TESTING REQUIRED.
- THE FOLLOWING ITEMS ARE SUBJECT TO SPECIAL INSPECTION IN CONFORMANCE WITH CBC SEC. 1703.1 (EXCEPTIONS MAY BE TAKEN WHERE APPLICABLE):
 - ALL STRUCTURAL WELDING.
 - CONCRETE WHEN DESIGN F_c > 2500 PSI (SEE CONCRETE MIN DESIGN SCHEDULE).
 - SPECIAL GRADING, EXCAVATION, AND FILLING.
 - INSTALLATION OF I.C.C. APPROVED WEDGE TYPE BOLTS INTO CONCRETE.
- SHOP DRAWING SUBMITTALS:
 - SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION. SEE SPECIFICATIONS FOR SUBMITTALS REQUIRED.
 - SHOP DRAWINGS SHALL NOT BE PREPARED UNTIL ALL CONDITIONS HAVE BEEN VERIFIED.
 - DETAILER SHALL SUBMIT PITS FOR ISSUES REQUIRING RESOLUTION FOR COMPLETION OF SHOP DRAWINGS. MINOR ISSUES MAY BE CLOSED IN THE SHOP DRAWINGS.
 - SHOP DRAWING PREPARATION SHALL INCLUDE A CONTINGENCY TO ALLOW FOR MINOR REVISIONS RESULTING FROM ARCHITECTS' AND ENGINEERS' REVIEW.
- DETAILS SHOWN IN THESE DRAWINGS FOR THE SUPPORT OF ROOF AND/OR FLOOR MOUNTED EQUIPMENT AND OPENINGS IN ROOF AND/OR FLOOR DECKS ARE TYPICAL CONDITIONS. CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND OTHER CONTRACT DOCUMENTS FOR EQUIPMENT AND OPENING LOCATIONS, SIZES AND MOUNTING REQUIREMENTS.

4. REINFORCED HOLLOW-UNIT CONCRETE BLOCK MASONRY

- MASONRY CONSTRUCTION SHALL BE OF GROUTED, REINFORCED, CONCRETE MASONRY UNITS WITH 75% AS LISTED ABOVE, REFER TO THE PROJECT SPECIFICATIONS FOR OTHER RELATED INFORMATION.
- ALL MASONRY WORK SHALL CONFORM TO THE 2016 CBC AND THE AISI C301-13 AND 530 1-13, ASCE 5-13 AND ASCE 6-13 RESPECTIVELY.
- SPECIAL INSPECTION SHALL BE PROVIDED BY AN INDEPENDENT INSPECTION AGENCY CERTIFIED FOR MASONRY INSPECTION, IN CONFORMANCE WITH AISI 530 LEVEL 4 SPECIAL INSPECTION TABLE 1.19.2.
- MATERIALS:
 - PORTLAND CEMENT ASTM C150 TYPE 1 OR 11, LOW ALKALI.
 - AGGREGATE - MORTAR SAND - ASTM C144; GROUT SAND - ASTM C404.
 - ADHESIVE: MORTAR - ASTM C201, TYPE S.
 - ADMITTERS - ONLY AS APPROVED BY THE ARCHITECT.
 - CONCRETE BLOCK MASONRY UNITS - ASTM C90, MIN. COMPRESSIVE STRENGTH AS INDICATED IN SCHEDULE BELOW BY UNIT STRENGTH METHOD WITH MAXIMUM .005% LINEAR SHRINKAGE WHEN TESTED IN ACCORDANCE WITH C.I.A. STANDARDS.
 - REINFORCING MATERIALS - ASTM A706, GRADE B, GROUTED REINFORCING BARS - WELDED REBAR - ASTM A706 (WHERE REQ'D PER PLANS AND DETAILS) SEE "MASONRY LAP SPLICES" FOR TYP REBAR LAP REQUIREMENTS.
- MORTAR:
 - ASTM C270 STRENGTH TYPE S, 1200 PSI @ 7 DAYS AND 1800 PSI @ 28 DAYS FOR LABORATORY PREPARED SAMPLES, 1500 PSI @ 28 DAYS FOR FIELD TEST SPECIMENS. FIELD SAMPLES SHALL BE USED TO VERIFY CONSISTENCY OF MATERIALS AND PROCEDURES ONLY.
- GROUT:
 - STRENGTH: AS REQUIRED PER SCHEDULE LISTED BELOW.
 - COMPOSITION: GROUT SHALL BE PROPORTIONED FOR "COARSE GROUT" PER ASTM C476/42 AND CBC 2103.3.
- ADMITTERS: ONLY AS APPROVED BY THE ARCHITECT.
- NO WELDING OF REINFORCING STEEL SHALL BE ALLOWED.
- LAP SPLICES: SEE SCHEDULE BELOW.
- COVER TO BARS: SEE SCHEDULE BELOW.
- CONCRETE CURING: SEE SPECIFICATIONS.
- FORM REMOVAL: SLICE FORMS OF FOOTINGS SLABS ON GRADE, MINIMUM 2 DAYS.
- VIBRATION: VIBRATE ALL CONCRETE IN PLACE WITH A MECHANICAL VIBRATOR USED BY EXPERIENCED PERSONNEL.
- DRILLED AND EXPOSED ANCHOR BOLTS WHERE ANCHOR BOLTS OR HOLDUP BOLTS ARE OMITTED, BOLTS SHALL BE SUBSTITUTED WITH DRILLED OR EXPOSED ANCHORS PER ENGINEERS WRITTEN DIRECTION.

3. CONCRETE

- GENERAL: ALL CONCRETE WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF THE ACI MANUAL OF CONCRETE PRACTICE AND THE C.B.C.
- REINFORCING MATERIALS:
 - DEFORMED ASTM A615 OR A706 - GRADE 60
 - WELDED WIRE FABRIC: ASTM A106
 - WELDED REBAR (IF USED): ASTM A706
- CONCRETE MIX DESIGNS: CONCRETE MIX SHALL BE LIMITED BY THE FOLLOWING. SEE SPECIFICATIONS FOR OTHER CONCRETE MIX INFORMATION.

LOCATION	COMP. STRENGTH (f' _c)	MINIMUM SACKS/YD ³	MAX. WATER/CEMENT RATIO	AGGREGATE SIZE
TYPICAL INTERIOR SLAB ON GRADE	4,000 psi (DESIGN=2,500 psi)	6 1/2 (15% FLYASH SUBSTITUTE REQUIRED)	.45	ASTM C33 SIZE 57
FOOTINGS	3,000 psi (SPECIAL INSPECTION)	5 1/2	.60	ASTM C33 SIZE 57
ENTRY TOWER FOOTING CORE	4,000 psi	6 (15% FLYASH SUBSTITUTE REQUIRED)	.45	ASTM C33 SIZE 8
EXTERIOR WALKWAYS & SITE WORK	SEE CIVIL	-	-	-
- ADMITTERS: ONLY AS APPROVED BY THE ARCHITECT.
- NO WELDING OF REINFORCING STEEL SHALL BE ALLOWED.
- LAP SPLICES: SEE SCHEDULE BELOW.
- COVER TO BARS: SEE SCHEDULE BELOW.
- CONCRETE CURING: SEE SPECIFICATIONS.
- FORM REMOVAL: SLICE FORMS OF FOOTINGS SLABS ON GRADE, MINIMUM 2 DAYS.
- VIBRATION: VIBRATE ALL CONCRETE IN PLACE WITH A MECHANICAL VIBRATOR USED BY EXPERIENCED PERSONNEL.
- DRILLED AND EXPOSED ANCHOR BOLTS WHERE ANCHOR BOLTS OR HOLDUP BOLTS ARE OMITTED, BOLTS SHALL BE SUBSTITUTED WITH DRILLED OR EXPOSED ANCHORS PER ENGINEERS WRITTEN DIRECTION.

LOADING DATA	ENTRY TOWER	SEISMIC DESIGN DATA	ENTRY TOWER
ROOF DEAD LOAD	28 psf	SITE COORDINATES	38,753 N -119,873 W
ROOF LIVE LOAD	20 psf	SEISMIC IMPORTANCE FACTOR (I)	1.0
WIND DESIGN DATA		RISK CATEGORY	II
ULTIMATE WIND SPEED (3 SECOND GUST)	110 mph	MAPPED SPECTRAL RESPONSE	S _s = .593 S ₁ = .246
WIND EXPOSURE CATEGORY	C	RISK CLASS	D
INTERNAL PRESSURE COEFFICIENT	± .18	SPECTRAL RESPONSE COEFFICIENTS	S _w = .024 S _u = .313
ANALYSIS PROCEDURE	ASCE CHAPTER 28	SEISMIC DESIGN CATEGORY	D
		SEISMIC RESISTING FORCE SYSTEMS	CANT. COL.
		SEISMIC RESPONSE COEFFICIENT(S) C _s	.420
		RESPONSE MODIFICATION FACTOR(S) R	1.25
		ANALYSIS PROCEDURE USED	ASCE 12.8 EQUIVALENT LATERAL FORCE

CONCRETE REINFORCEMENT COVER

LOCATION	MINIMUM COVER
CONCRETE CAST AND PERMANENTLY EXPOSED TO EARTH:	3"
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#8 THROUGH #18 BAR	2"
#5 BAR, W31 OR D31, AND SMALLER	1 1/2"
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
#14 AND #18 BAR	1 1/2"
#10 BAR AND SMALLER	1 1/4"

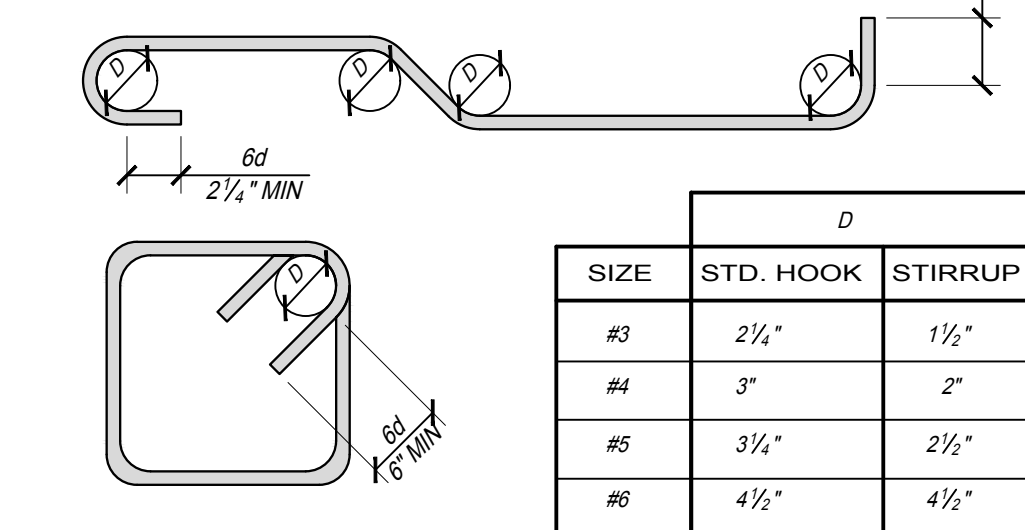
CONCRETE REINFORCEMENT LAP SPLICES

- MIN. SPLICES UNLESS OTHERWISE DIMENSIONED ON DRAWINGS:
- | CONCRETE BAR TYPES | LAP TYPE |
|------------------------------------|----------|
| FOOTING BARS (OTHER THAN TOP BARS) | CL1 |
| HORIZ. & VERT. WALL BARS | CL2 |
| FOOTING TOP BARS | CL2 |
- REINFORCING STEEL SHALL BE PROPERLY PLACED AND SHALL NOT BE RELOCATED TO ACCOMMODATE CONDITIONS.
- GROUT COVER BETWEEN ANCHOR AND REINFORCING STEEL SHALL BE 2.5 x BAR DIAMETER, 1/2" MIN. (1/2" AT #6 BARS, 1/4" AT #6 BARS).
- MAINTAIN A MINIMUM CLEAR AREA WITHIN THE CELL FOR GROUT CLEARANCE AND CONSOLIDATION BY VIBRATION.
- UNREINFORCED CELLS: LIMIT CONDUIT TO TWO 1/2" DIAMETER CONDUIT PER CELL, OR ONE 1/2" DIAMETER CONDUIT PER CELL PROVIDED THE FOLLOWING CONDITIONS ARE MAINTAINED:
- CONDUIT SHALL NOT BE PLACED CLOSER THAN 3 x DIAMETER, CENTER TO CENTER, TO ADJACENT CONDUITS.
 - CONDUIT SHALL NOT BE PLACED CLOSER THAN 3 x DIAMETER, CENTER TO CENTER, TO ADJACENT CONDUITS.
 - MAINTAIN A MINIMUM CLEAR AREA WITHIN THE CELL FOR GROUT CLEARANCE AND CONSOLIDATION BY VIBRATION.
 - NO CONDUITS ARE ALLOWED IN WALLS LESS THAN 8" NOMINAL THICKNESS.

BAR SIZE	CL1	CL2	CL3
#4	24"	30"	48"
#5	30"	36"	60"
#6	40"	48"	72"



REINFORCEMENT BENDING REQUIREMENTS



SIZE	STD. HOOK	STIRRUP
#3	2 1/2"	1 1/2"
#4	3"	2"
#5	3 1/2"	2 1/2"
#6	4 1/2"	4 1/2"

1. GENERAL NOTES

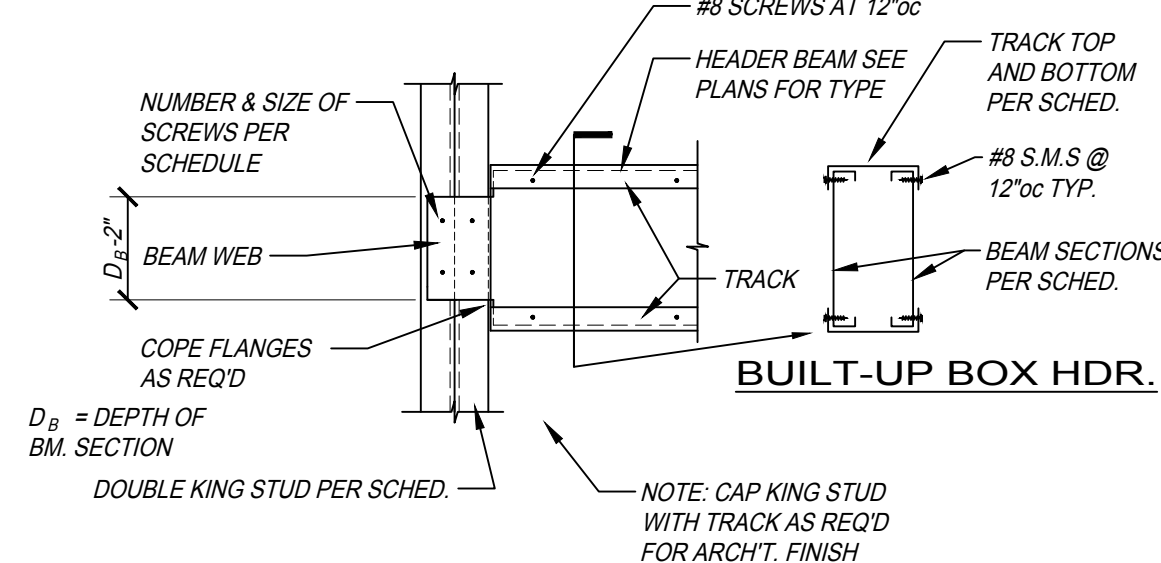
- ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE SECTIONS OF THE CALIFORNIA BUILDING CODE (CBC), 2016 EDITION, AND ALL OTHER PUBLICATIONS AND STANDARDS LISTED HEREIN.
- ALL DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND ALL OTHER CONTRACT DRAWINGS AND SPECIFICATIONS.
- DETAILS SHOWN ON STRUCTURAL DRAWINGS ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS.
- DIMENSIONS SHOWN SHALL TAKE PRECEDENCE OVER SCALE ON PLANS, SECTIONS AND DETAILS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT IMMEDIATELY.
- NOTES AND DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
- FRAMING AND DETAIL CONDITIONS SPECIFIED BY THESE DRAWINGS SHALL NOT BE MODIFIED WITHOUT WRITTEN DOCUMENTATION FROM THE ENGINEER AND ARCHITECT.
- CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FLOOR OR ROOF FRAMING MEMBERS. LOAD SHALL NOT EXCEED DESIGN LIVE LOAD.
- DESIGN LOADING: PER CBC, 2016 EDITION.
- CONSTRUCTION DOCUMENTS SHALL CONSIST OF THE "APPROVED" DRAWINGS, SPECIFICATIONS AND ADDENDUM BEARING THE STAMP AND SIGNATURE OF THE ARCHITECT AND THE APPROVAL STAMP OF THE JURISDICTIONAL BUILDING DEPARTMENT. STRUCTURAL CALCULATIONS ARE NOT PART OF THE CONSTRUCTION DOCUMENTS AND SHALL NOT BE USED FOR CONSTRUCTION PURPOSES.
- ALL WORK SHALL BE PERFORMED FROM THE "APPROVED" DOCUMENTS ONLY. A FULL SET OF APPROVED DOCUMENTS SHALL BE KEPT ON SITE DURING ALL CONSTRUCTION PHASES.
- DESIGN DATA CONDITIONS AS LISTED BELOW.

LOADING DATA	ENTRY TOWER	SEISMIC DESIGN DATA	ENTRY TOWER
ROOF DEAD LOAD	28 psf	SITE COORDINATES	38,753 N -119,873 W
ROOF LIVE LOAD	20 psf	SEISMIC IMPORTANCE FACTOR (I)	1.0
WIND DESIGN DATA		RISK CATEGORY	II
ULTIMATE WIND SPEED (3 SECOND GUST)	110 mph	MAPPED SPECTRAL RESPONSE	S _s = .593 S ₁ = .246
WIND EXPOSURE CATEGORY	C	RISK CLASS	D
INTERNAL PRESSURE COEFFICIENT	± .18	SPECTRAL RESPONSE COEFFICIENTS	S _w = .024 S _u = .313
ANALYSIS PROCEDURE	ASCE CHAPTER 28	SEISMIC DESIGN CATEGORY	D
		SEISMIC RESISTING FORCE SYSTEMS	CANT. COL.
		SEISMIC RESPONSE COEFFICIENT(S) C _s	.420
		RESPONSE MODIFICATION FACTOR(S) R	1.25
		ANALYSIS PROCEDURE USED	ASCE 12.8 EQUIVALENT LATERAL FORCE

2. SITE PREP. & FOUNDATION

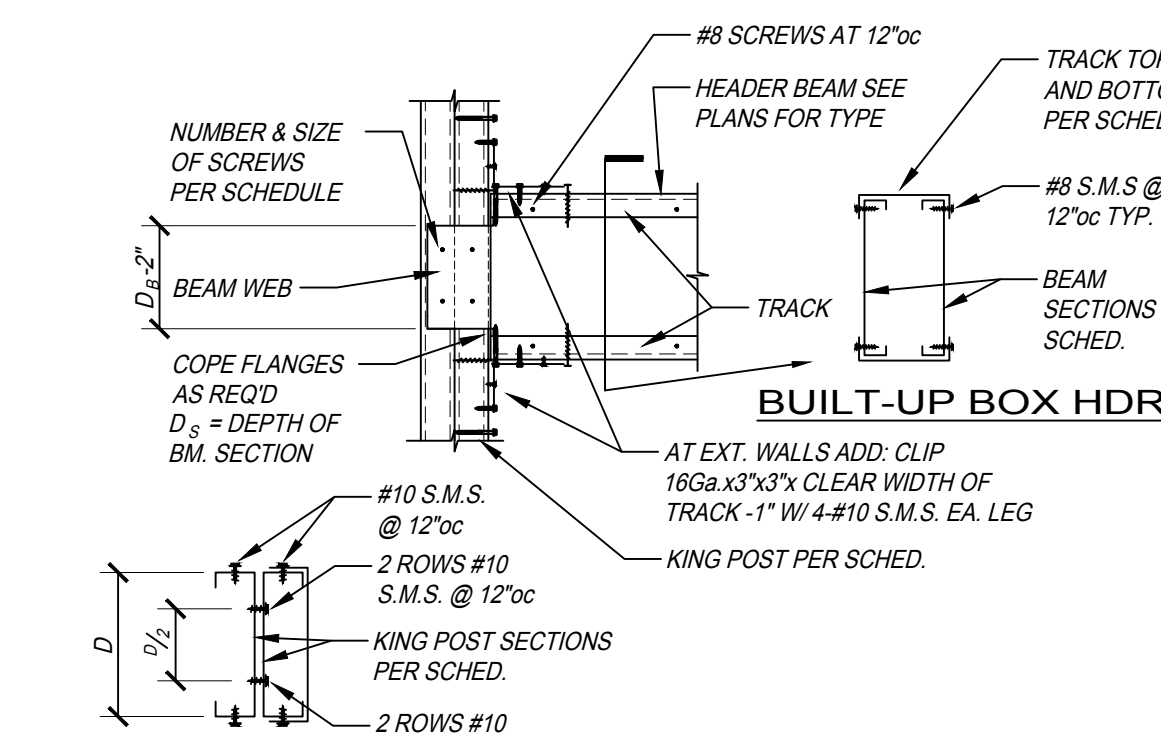
- FOUNDATION DESIGN: BASED ON ALLOWABLE SOIL BEARING PRESSURES AND OTHER REQUIREMENTS PER SOIL REPORT, YES NO. 190986.01 BY TECHCON ENGINEERING SERVICES, DATED 11-01-2016.
 - ALLOWABLE BEARING PRESSURES:
 - STATIC DEAD + LIVE: 3000 psf
 - COMBINED (DEAD + LIVE + SEISMIC): 3000 psf
 - ACTIVE PRESSURE: 90 psf
 - AT-RISK PRESSURE: 90 psf
 - PASSIVE PRESSURE: 128 psf STATIC
172 psf COMBINED
0.25 STATIC
0.24 COMBINED
 - FRICITION COEFFICIENT: 0.24 COMBINED
- COMPACTION REQUIREMENTS: REFER TO THE SOILS REPORT.
- ENGINEERING FILL: REFER TO THE SPECIFICATIONS AND SOILS REPORT. ALL ENGINEERED FILL SHALL BE SUBJECT TO "SPECIAL INSPECTION" AS REQUIRED BY THE ARCHITECT AND THE LOCAL BUILDING OFFICIALS.
- REFER TO THE ARCHITECT'S DRAWINGS FOR FINISHED FLOOR ELEVATIONS.
- ALL FOOTINGS SHALL EXTEND TO FIRM BEARINGS. MINIMUM FOOTING EMBEDMENT = 18" BELOW LOWEST ADJACENT GRADE.
- SEE ARCHITECT'S & CIVIL DRAWINGS FOR SIZE AND LOCATION OF NON-BEARING PARTITIONS.
- SEE ARCHITECT'S DRAWINGS FOR EXTENT OF EXTERIOR WALKWAYS.
- THE LOCATIONS OF CONSTRUCTION AND CONTROL JOINTS (C.J.) ARE THE CONTRACTOR'S RESPONSIBILITY FOR THE CONTROL OF CONCRETE SLAB CRACKING WITHIN THE RECOMMENDED LIMITATIONS AS FOLLOWS:
 - JOINTS (C.J.) ARE TO BE PROVIDED TO BREAK THE FLOOR INTO WORKING AREAS NOT LARGER THAN 60' x 71'
 - JOINTS SHALL BE SPACED NOT MORE THAN 25' o.c. WHERE POSSIBLE.
 - JOINTS SHALL BE LOCATED AS TO NOT EXCEED A LENGTH TO WIDTH RATIO OF 1:25 WITHIN JOINTED AREAS.
 - JOINTS SHALL BE LOCATED WITH CONSIDERATION OF THE CRACK POTENTIAL OF INSIDE CORNERS AT SLAB EDGES.
 - JOINTS SHALL BE LOCATED BELOW INTERIOR PARTITION WALLS UNLESS NOTED OTHERWISE.
 - JOINT LOCATIONS SHALL BE REVIEWED AND ACCEPTED BY THE ARCHITECT PRIOR TO POURING SLABS.

INTERIOR/NON-STRUCTURAL WALL HEADER BEAM SCHEDULE					
CONN. TYPE	TRACKS (X=WALL WIDTH)	UNPUNCHED BEAM	BEAM SPAN	SCREWS	KING STUD (X=WALL WIDTH)
A'	X007150-33	800S162-33	6'-0"	4-#10 (2 EA. SIDE)	1-X00S162-33
B'	X007150-33	800S162-33	10'-0"	8-#10 (4 EA. SIDE)	2-X00S162-33
C'	X007150-33	1000S162-54	12'-0"	12-#10 (6 EA. SIDE)	2-X00S162-54

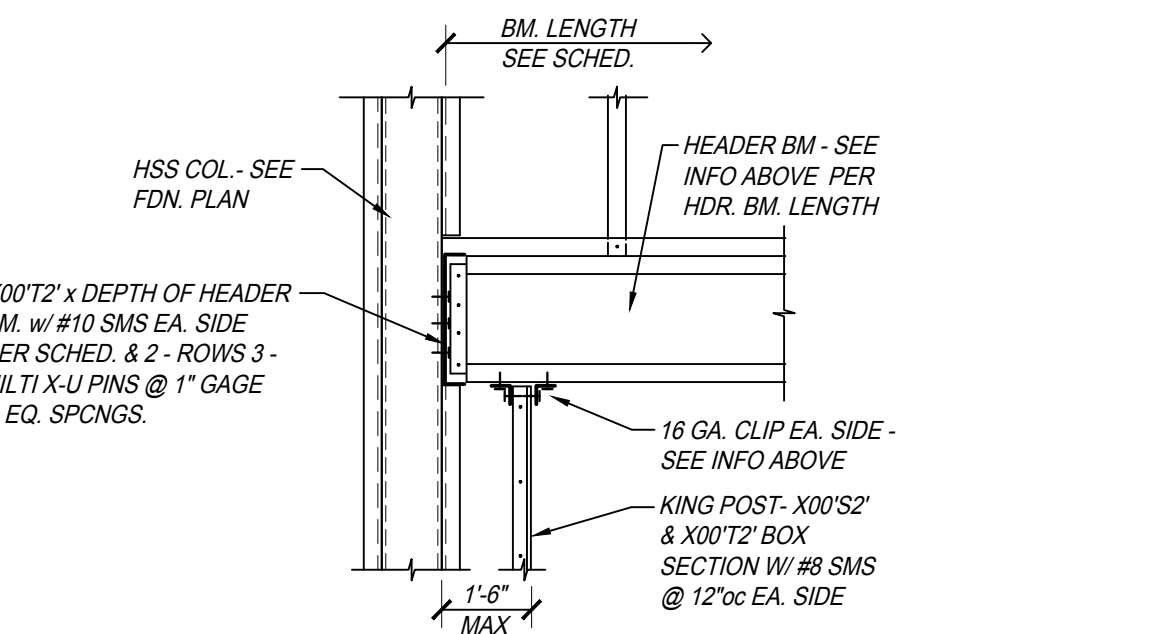


EXTERIOR & INTERIOR BEARING WALL HEADER BEAM CONNECTION

HEADER TYPE	TRACKS (X=WALL WIDTH)	UNPUNCHED BEAM	MAX. BEAM LENGTH	SCREWS	KING POST
EH-1	600T150-54	800S162-54	10'-8"	6-#10 (3 EA. SIDE)	1-600T150-54 2-600S162-54
EH-2	600T150-43	600S162-54	5'-4"	4-#10 (4 EA. SIDE)	1-600T150-54 1-600S162-54

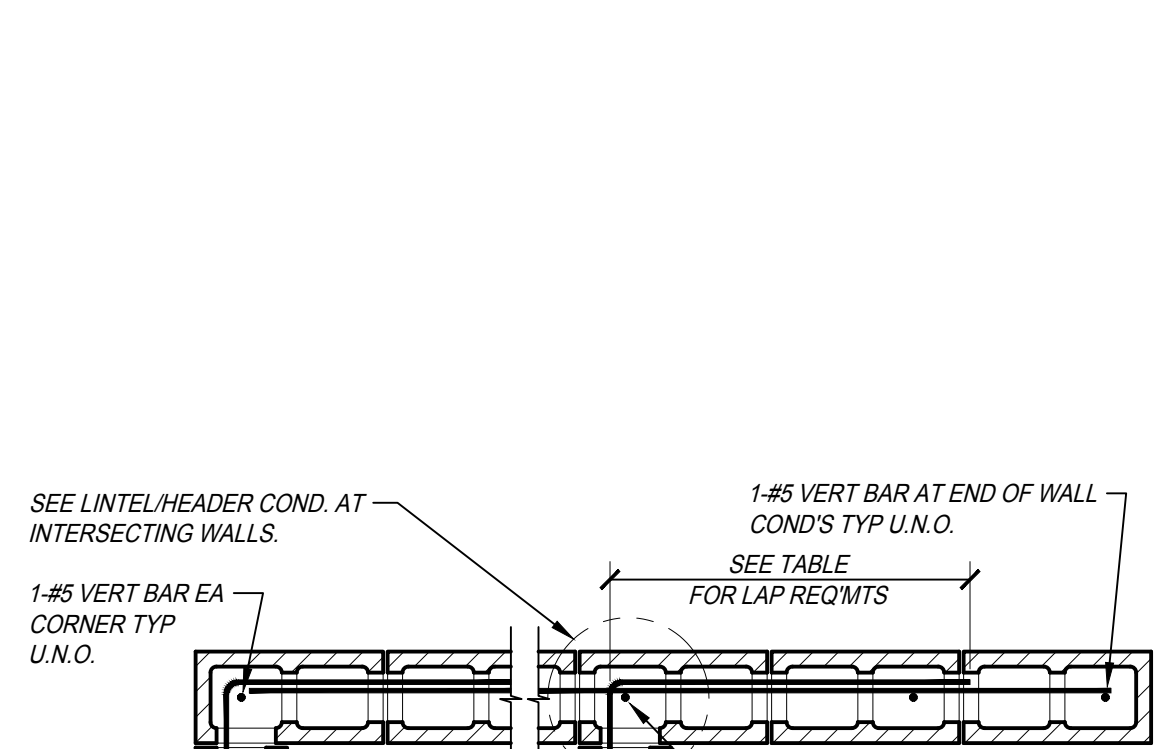


EXTERIOR WALLS



DETAIL

SCALE: 3/4" = 1'-0" TMET11 S1.1



MASONRY LAP SPLICES

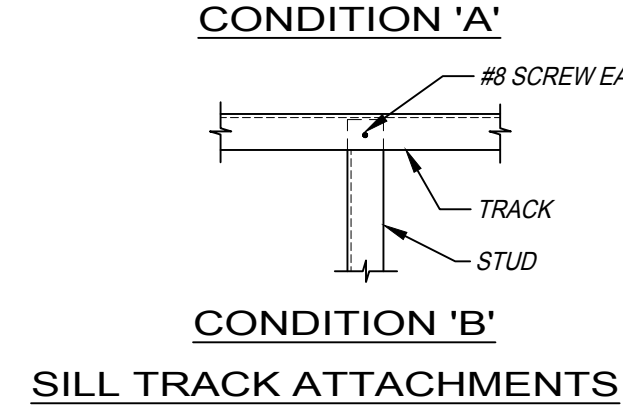
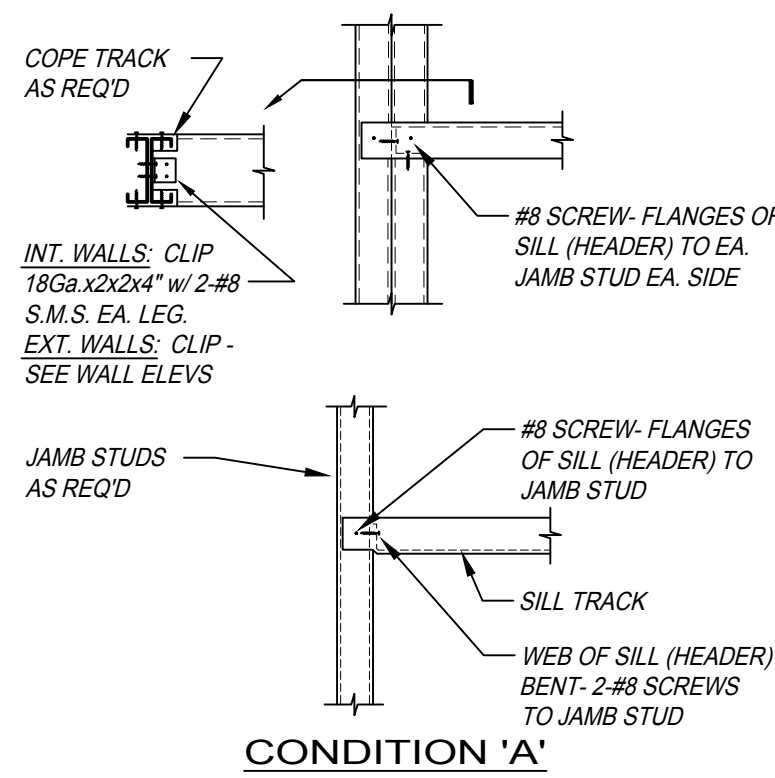
BAR SIZE	SINGLE LAYER REINF	
	MIN COVER	MIN LAP LENGTH
#4	3"	24"
#5	3"	28"

- LAP SPLICES SHOWN ARE FOR:
- WALL HORIZONTAL BARS
 - WALL VERTICAL BARS
 - WALL DOWELS TO FOOTING

TYPICAL BLOCK REINFORCEMENT AT WALL INTERSECTIONS

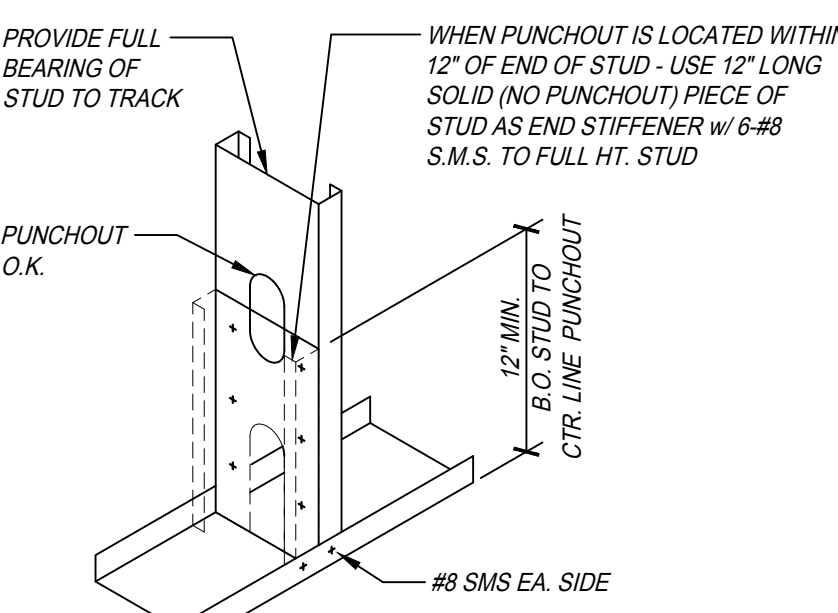
DETAIL

SCALE: 3/4" = 1'-0" TMSAC S1.1



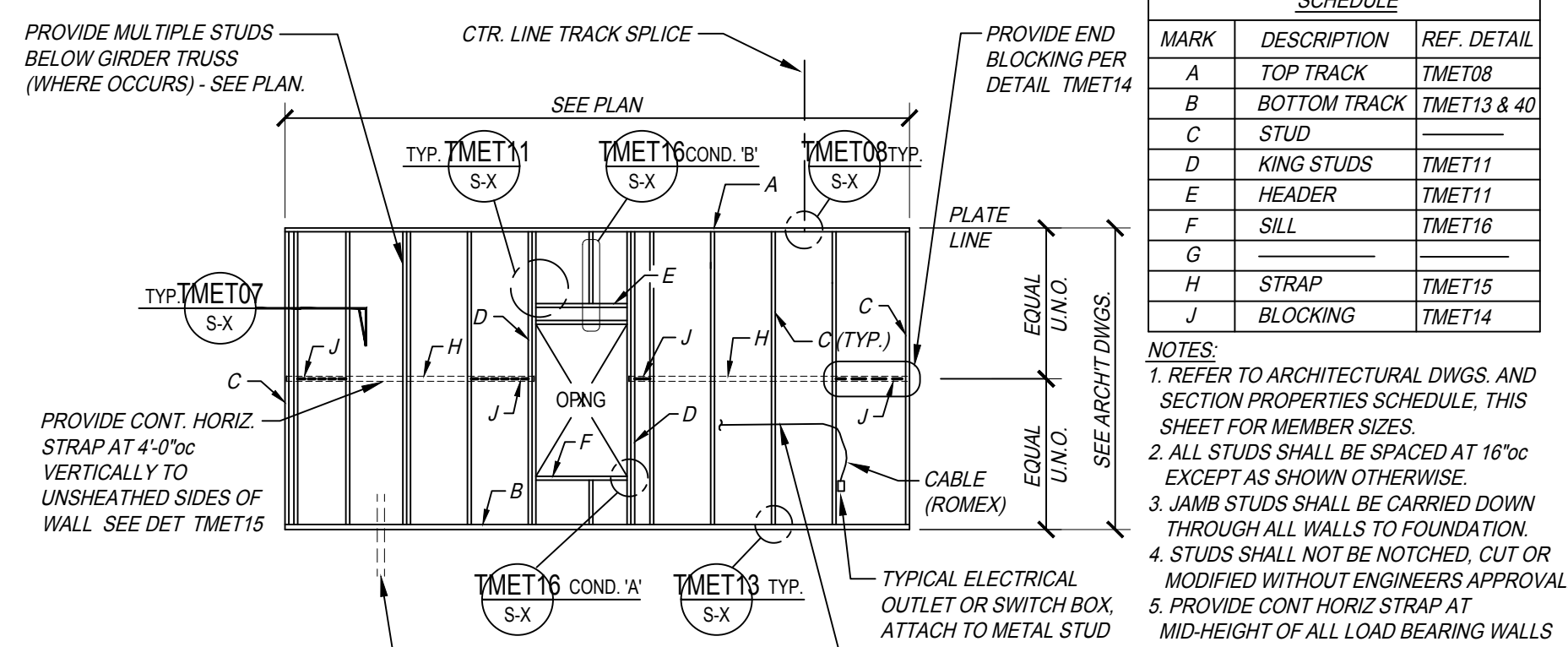
DETAIL

SCALE: 3/4" = 1'-0" TMET10 S1.1



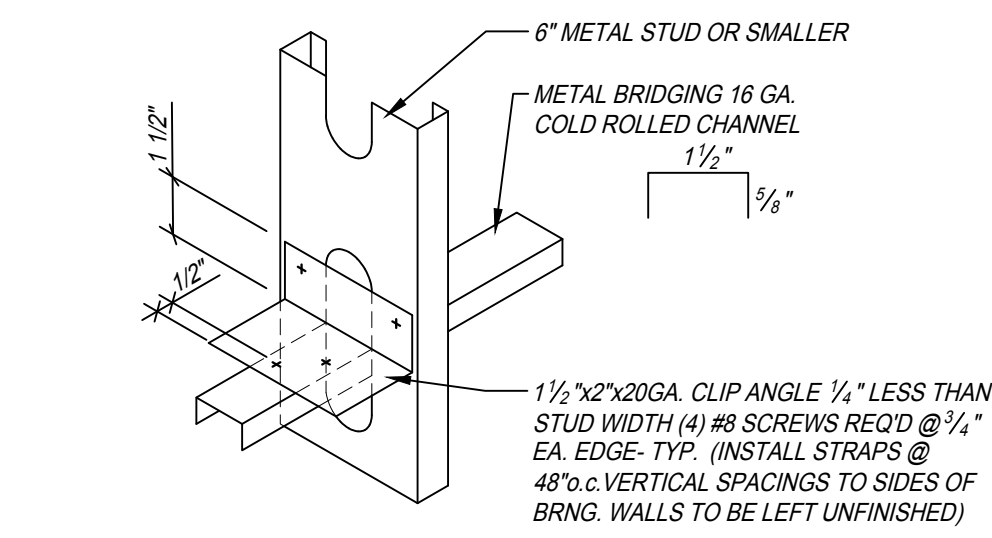
DETAIL

SCALE: 3/4" = 1'-0" TMET13 S1.1

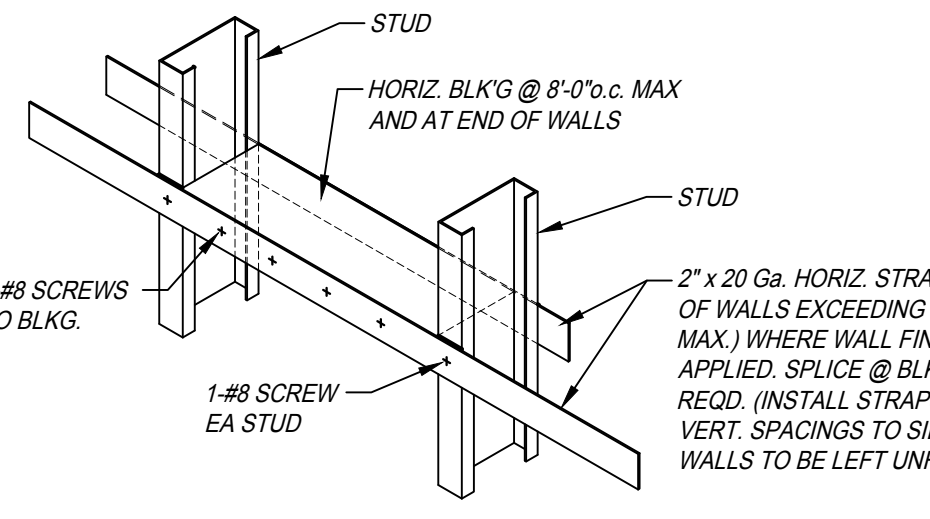


DETAIL

SCALE: 3/4" = 1'-0" TMET10-DSA S1.1



DETAIL A

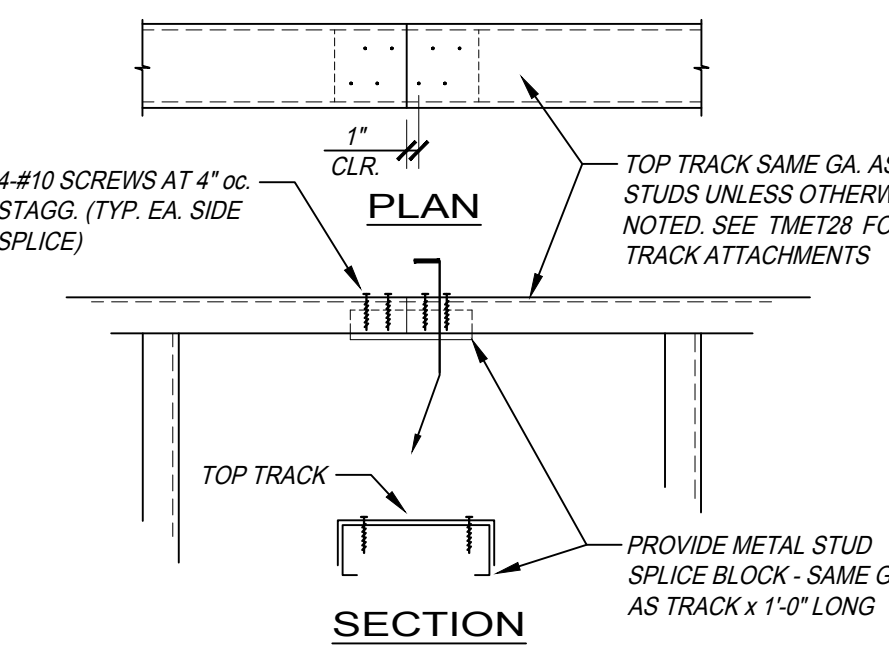


DETAIL

SCALE: 3/4" = 1'-0" TMET15 S1.1

DETAIL

SCALE: 3/4" = 1'-0" TMET07 S1.1

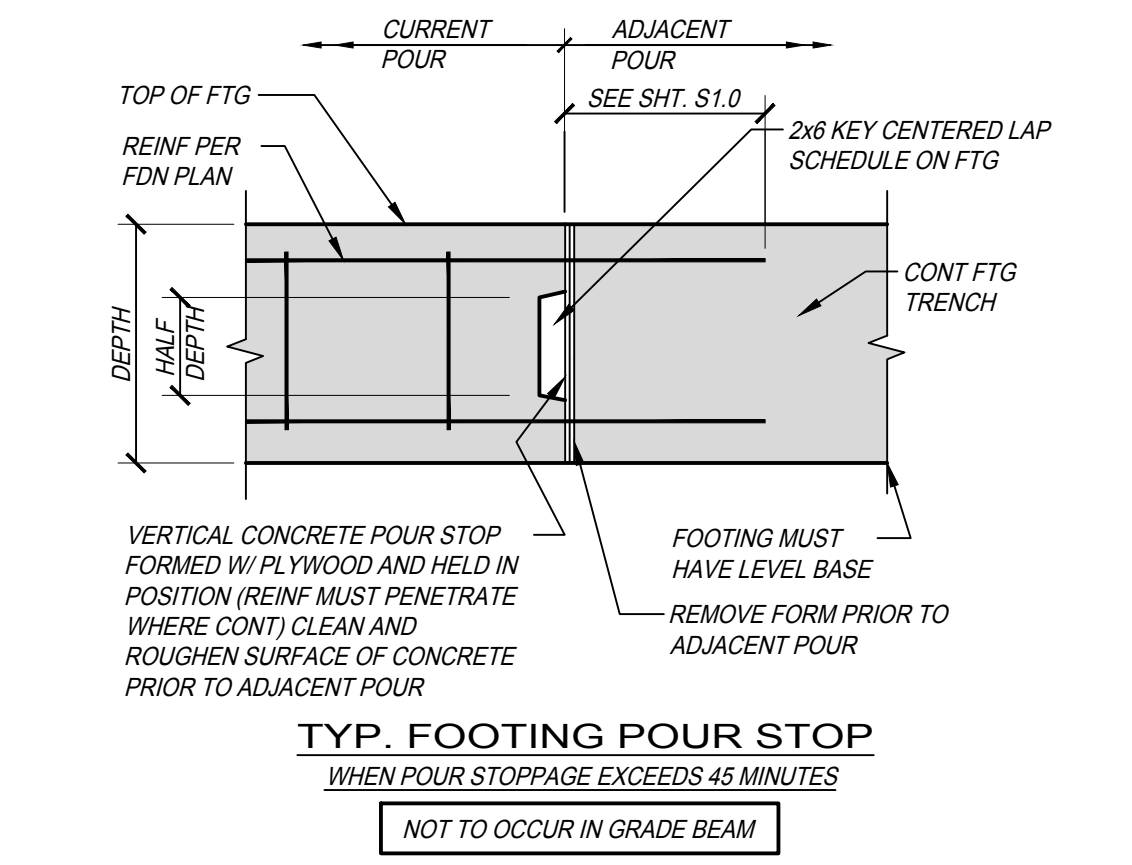


DETAIL

SCALE: 3/4" = 1'-0" TMET08 S1.1

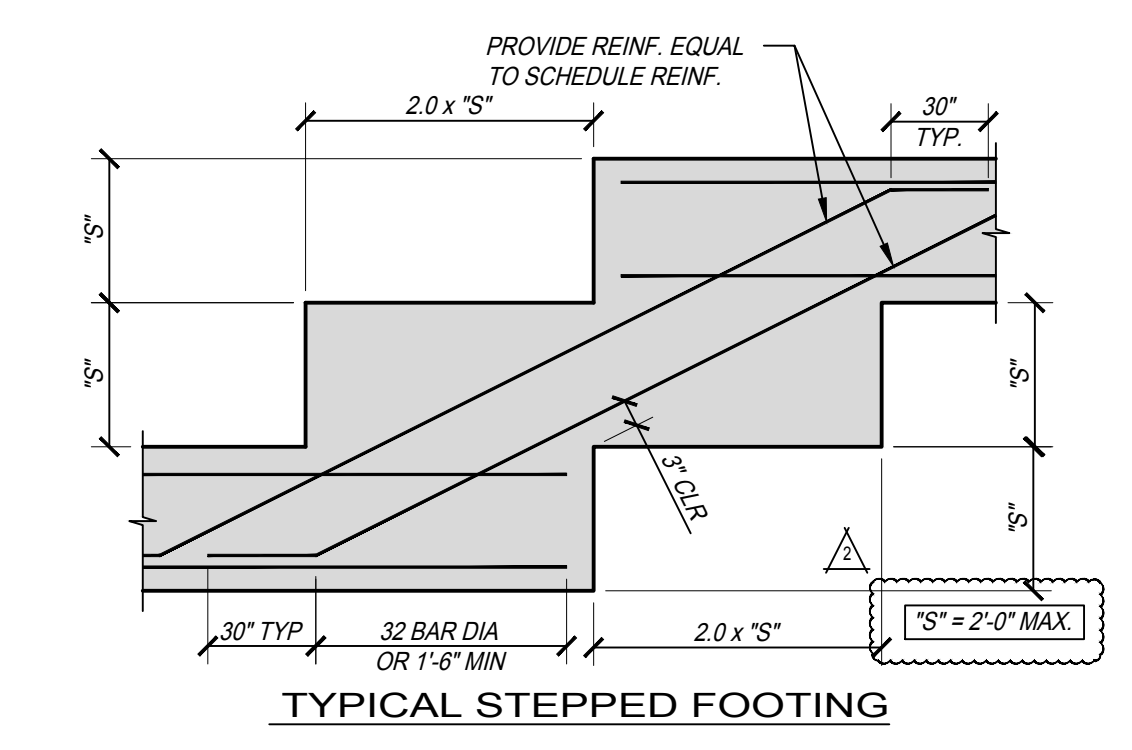
SCHEDULE		
MARK	DESCRIPTION	REF. DETAIL
A	TOP TRACK	TMET08
B	BOTTOM TRACK	TMET13 & 40
C	STUD	
D	KING STUDS	TMET11
E	HEADER	TMET11
F	SILL	TMET16
G		
H	STRAP	TMET15
J	BLOCKING	TMET14

- NOTES:
- REFER TO ARCHITECTURAL DWGS. AND SECTION PROPERTIES SCHEDULE. THIS SHEET FOR MEMBER SIZES.
 - ALL STUDS SHALL BE SPACED AT 16"oc EXCEPT AS SHOWN OTHERWISE.
 - JAMB STUDS SHALL BE CARRIED DOWN THROUGH ALL WALLS TO FOUNDATION.
 - STUDS SHALL NOT BE NOTCHED, CUT OR MODIFIED WITHOUT ENGINEERS APPROVAL.
 - PROVIDE CONT. HORIZ. STRAP AT MID-HEIGHT OF ALL LOAD BEARING WALLS.



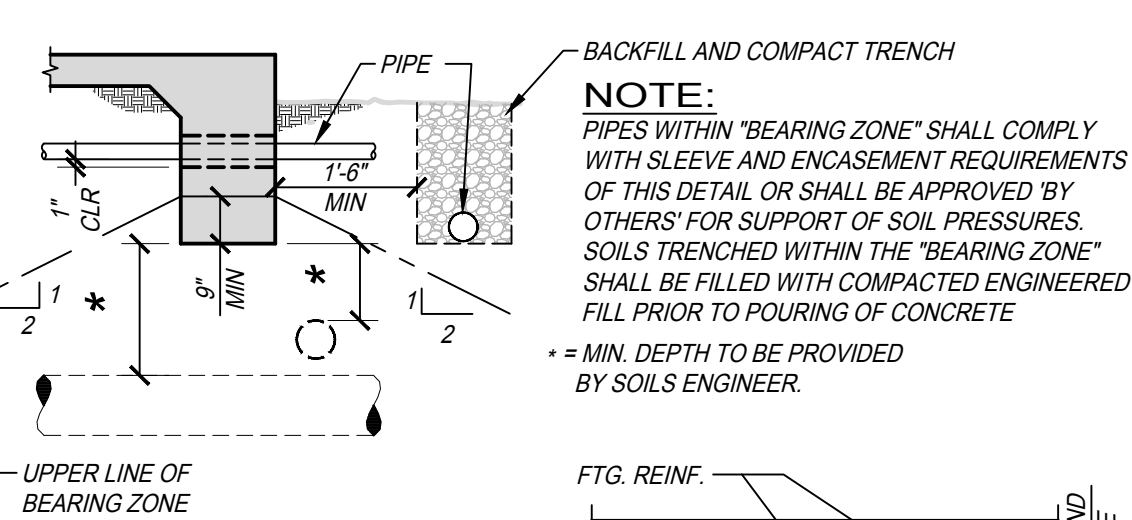
DETAIL

SCALE: 3/4" = 1'-0" TFDNA S1.1



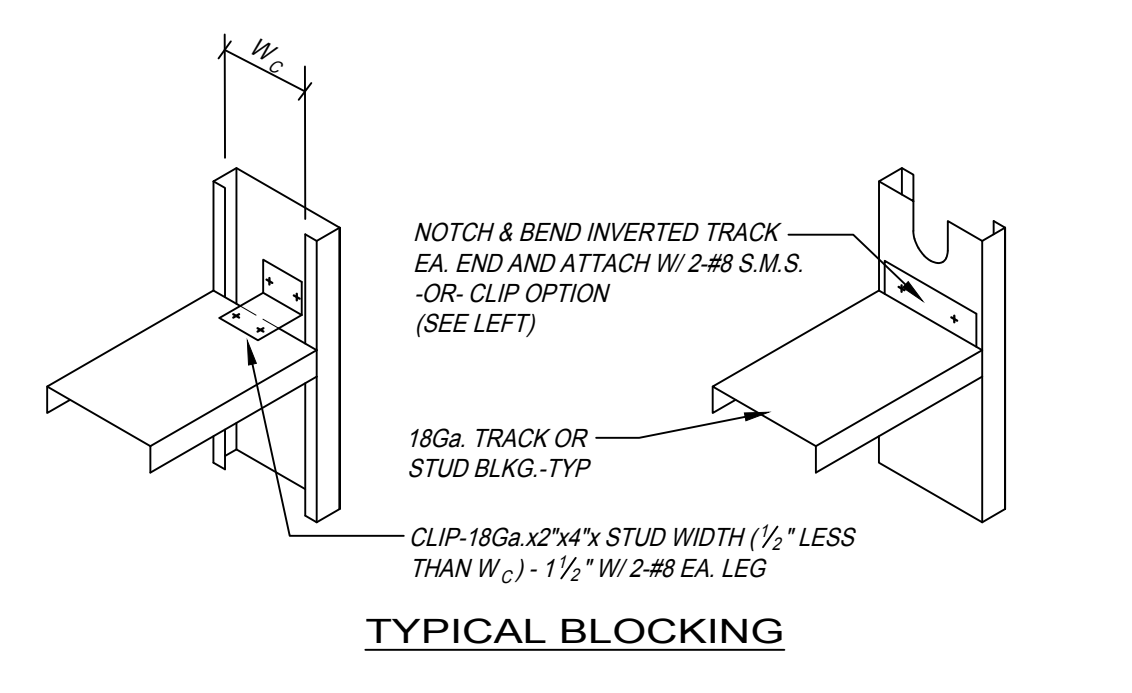
DETAIL

SCALE: 3/4" = 1'-0" TFDNB S1.1



DETAIL

SCALE: 3/4" = 1'-0" TFDN07 S1.1

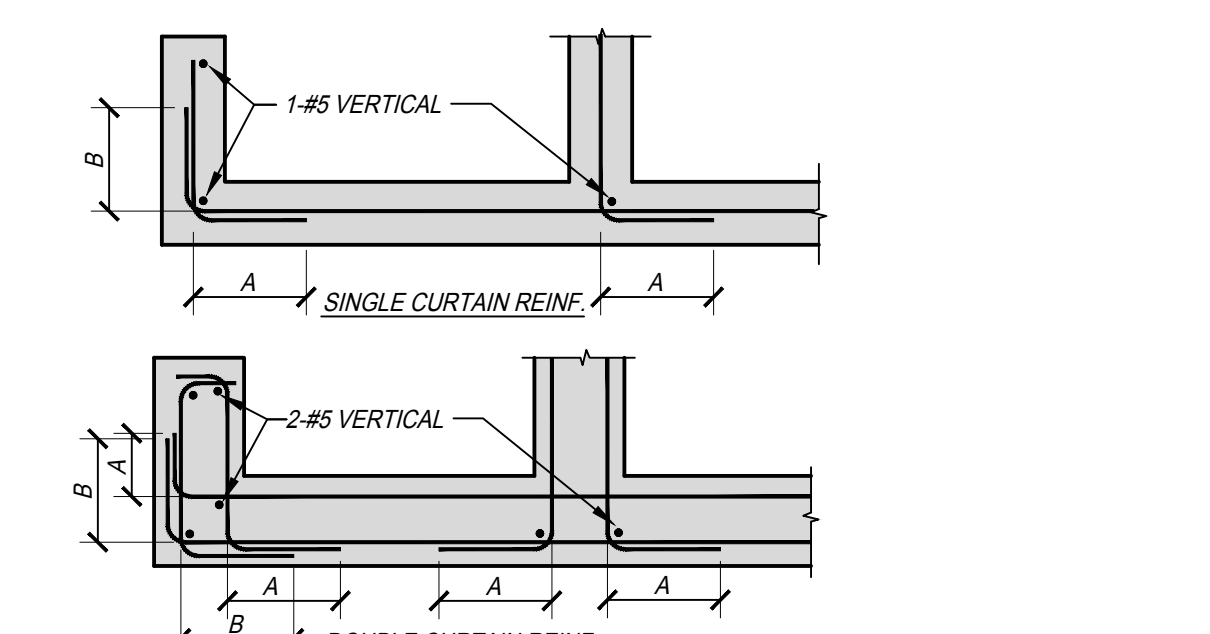


DETAIL

SCALE: 3/4" = 1'-0" TMET14 S1.1

DETAIL

SCALE: 3/4" = 1'-0" TFDN03 S1.1



BAR SIZE	#4	#5	#6	#7	#8	#9
GRADE 40	1'-6"	1'-6"	1'-6"	1'-10"	2'-5"	3'-0"
	B	1'-9"	1'-9"	2'-4"	3'-2"	4'-2"
GRADE 60	A	1'-6"	1'-6"	2'-0"	2'-9"	3'-7"
	B	1'-9"	2'-6"	3'-6"	4'-9"	7'-8"

DETAIL

SCALE: 3/4" = 1'-0" TFDN03 S1.1

6-16-2020

REGISTERED ARCHITECT
STATE OF CALIFORNIA
No. 52331
Exp. 3/21
R/17/152
STRUCTURAL
C. 27818
RES. 10-31-19

Project:
Sheriff Area 2 Sub-Station
1129 N. Armstrong Ave., Fresno, CA
APN: 310-133-04, 05, and 06
ISSUE DATE: 06.17.2020
PROJECT NO.: 180293 | 190203
FILE NAME: S1.0 - Substation

Sheet Content:
TYPICAL DETAILS

Fresno County Department of Public Works and Planning
Capital Projects

2220 Tulare Street, 8th Floor
Fresno, California 93721

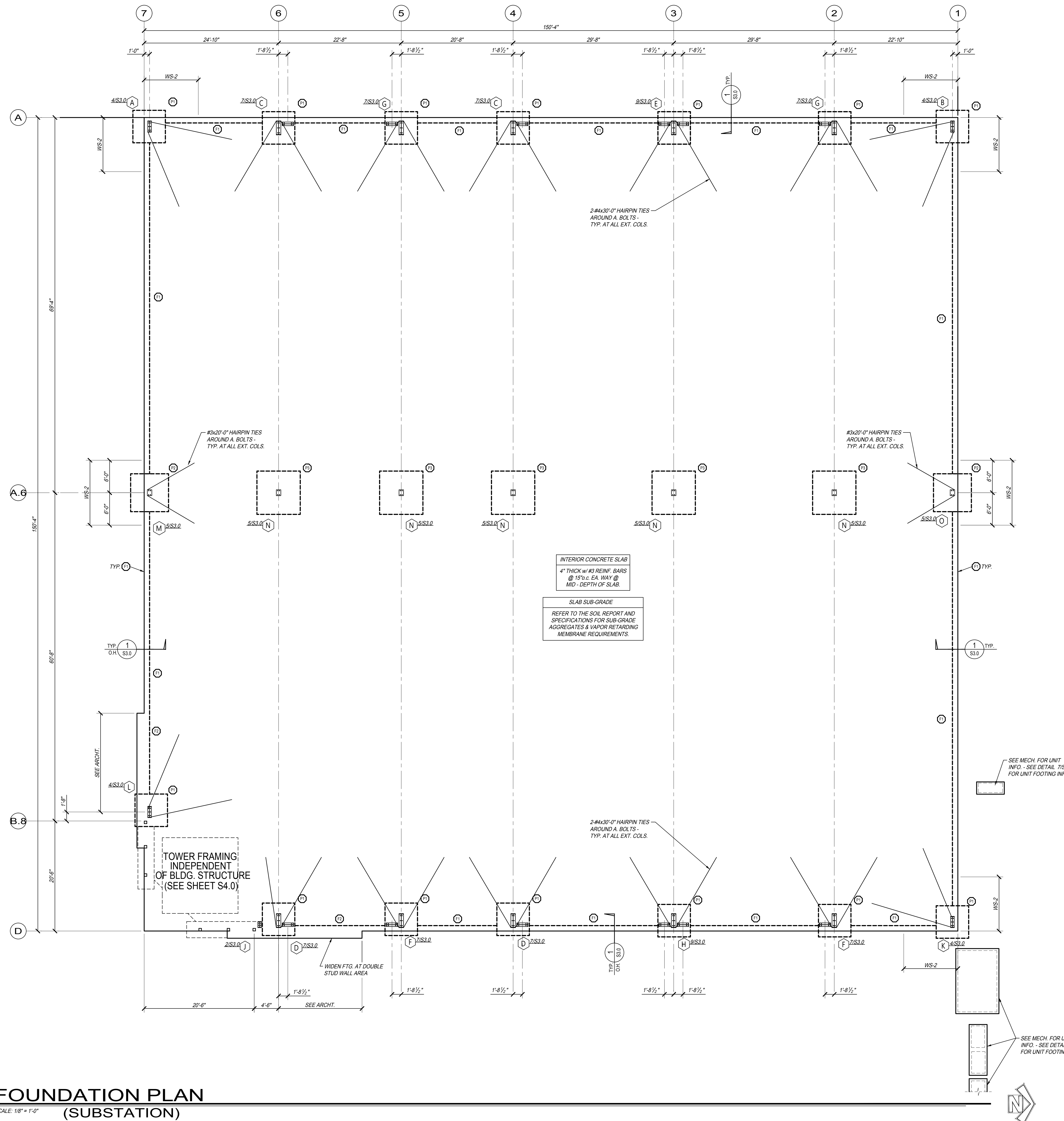
Sheet No.

S1.1

Drawn by: SMP Plot date: 06.17.2020

PARRISH HANSEN
STRUCTURAL ENGINEERS
A Division of Povost & Pritchard Consulting Group
418 CLOVIS AVE. ■ CLOVIS, CA 93612
PHONE 559.323.1023 ■ FAX 559.323.8090
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FOUNDATION PLAN
(SUBSTATION)

SCALE: 1/8" = 1'-0"

FOUNDATION NOTES

- REFER TO GENERAL NOTES AND TYPICAL DETAILS ON S.10.
- ALL EMBEDDED ITEMS SHALL BE IN PLACE & SECURED PRIOR TO POURING OF CONCRETE.
- ⊖ = FOOTING TYPE - SEE "FOOTING SCHEDULE"
- OR H = STEEL COLUMN

FOOTING SCHEDULE		
TYPE	SIZE	REINFORCEMENT
⊖	1'-0" WIDE x 1'-6" DEEP	2-#5 CONT. TOP 2-#5 CONT. BOTTOM
⊖	2'-4" WIDE x 1'-6" DEEP	3-#5 CONT. TOP & BOT. #4 TIES @ 24" O.C.
⊖	8'-0" SQUARE x 2'-0" DEEP	6-#5 EA. WAY (TOP & BOTTOM)
⊖	7'-0" SQUARE x 2'-0" DEEP	7-#5 EA. WAY (TOP & BOTTOM)
⊖	8'-0" SQUARE x 2'-0" DEEP	8-#5 EA. WAY (TOP & BOTTOM)

NOTES:
 1. ALL FOOTINGS SHALL EXTEND A MINIMUM OF 1'-6" INTO NATIVE SOIL.
 2. SEE DETAILS FOR FOOTING SIZE AND REINF. REQUIRED AT ALL RETAINING WALLS.
 3. FOOTING ARE TYPE ⊖ UNLESS NOTED OTHERWISE.

EXTERIOR WALL STUD SCHEDULE			
TYPE	SIZE	SPACING	SILL & TOP TRACKS
WS-1	800 'S3'	16" O.C.	800 'T2' OR 'T3'
WS-2	600 'S3'	8" O.C.	800 'T2' OR 'T3'

NOTE: ALL EXT. STUD WALLS ARE TYPE WS-1 UNLESS NOTED.

INTERIOR CONCRETE SLAB
4" THICK w/ #3 REINF. BARS @ 15" O.C. EA. WAY @ MID-DEPTH OF SLAB.

SLAB SUB-GRADE
REFER TO THE SOIL REPORT AND SPECIFICATIONS FOR SUB-GRADE AGGREGATES & VAPOR RETARDING MEMBRANE REQUIREMENTS.

TOWER FRAMING INDEPENDENT OF BLDG. STRUCTURE (SEE SHEET S4.0)

6-16-2020

REGISTERED ARCHITECT
NOEL ROGER DAVIDSON
California Licensed Architect No. C-27818
Plan: 10-31-2019
Fresno County Department of Public Works
Capital Projects
2201 Tulare Street, Eighth Floor
Fresno, California 93721
Telephone: (559) 600-4477
E-mail: ndavidson@co.fresno.ca.us

Project:
Sheriff Area 2 Sub-Station
1129 N. Armstrong Ave., Fresno, CA
APN: 310-133-04, -05, and -06
ISSUE DATE: 06.17.2020
PROJECT NO.: 180293 | 19023
FILE NAME: S2.0 - Substation

Sheet Content:
FOUNDATION PLAN

REGISTERED PROFESSIONAL ENGINEER
ROBERT S. PARRISH
No. 52331
Exp. 3/21
R/S/17/52
STRUCTURAL
STATE OF CALIFORNIA

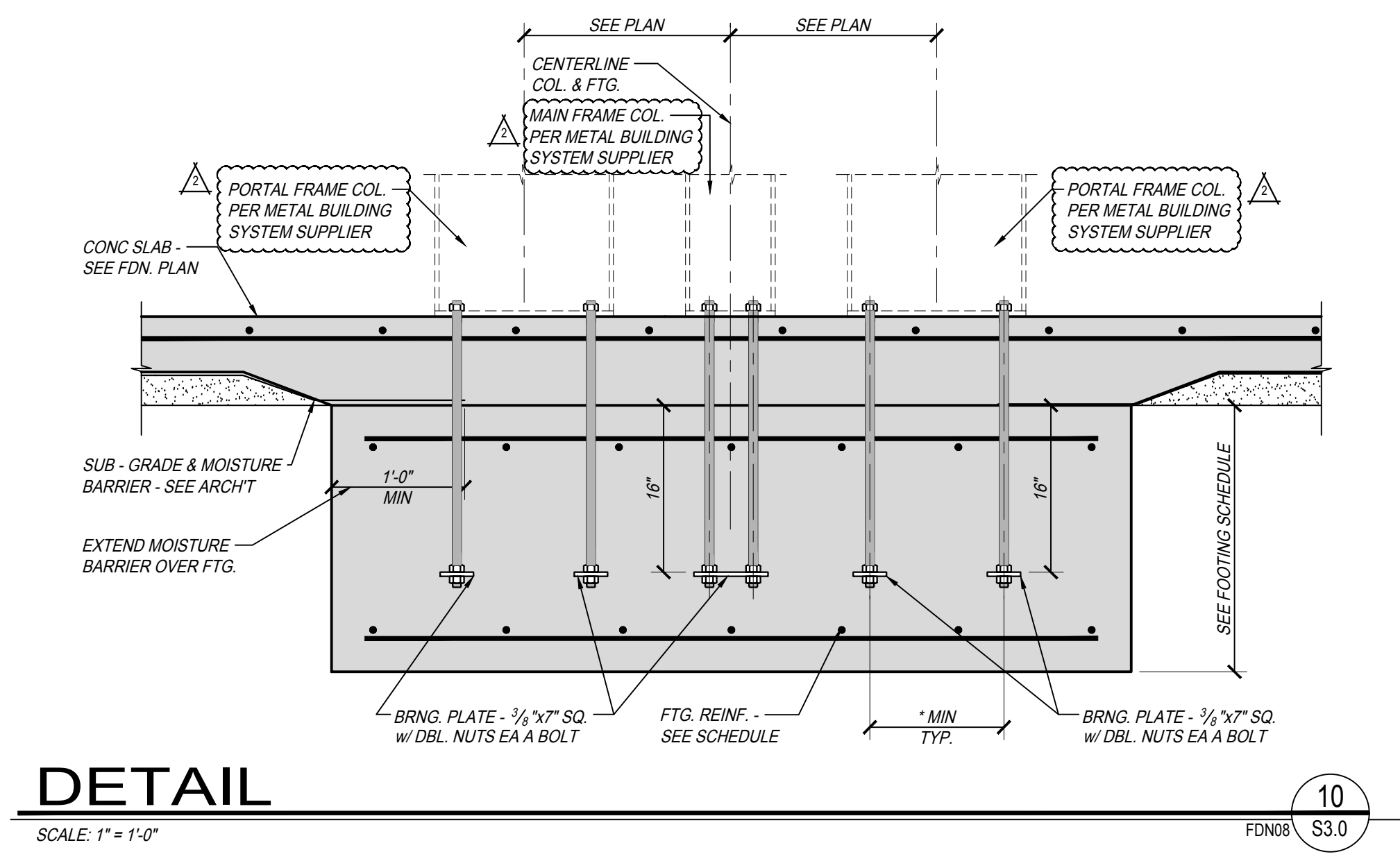
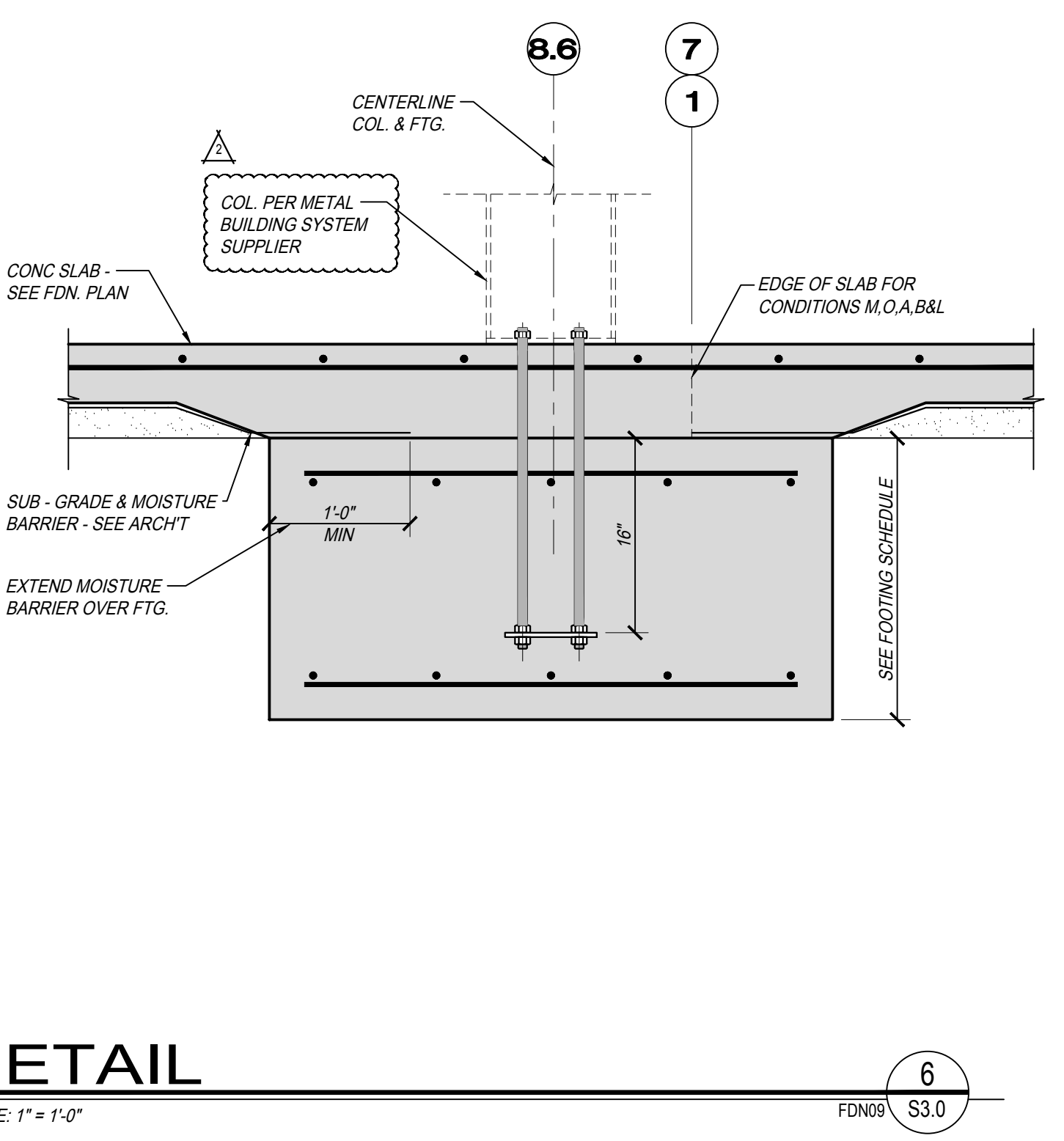
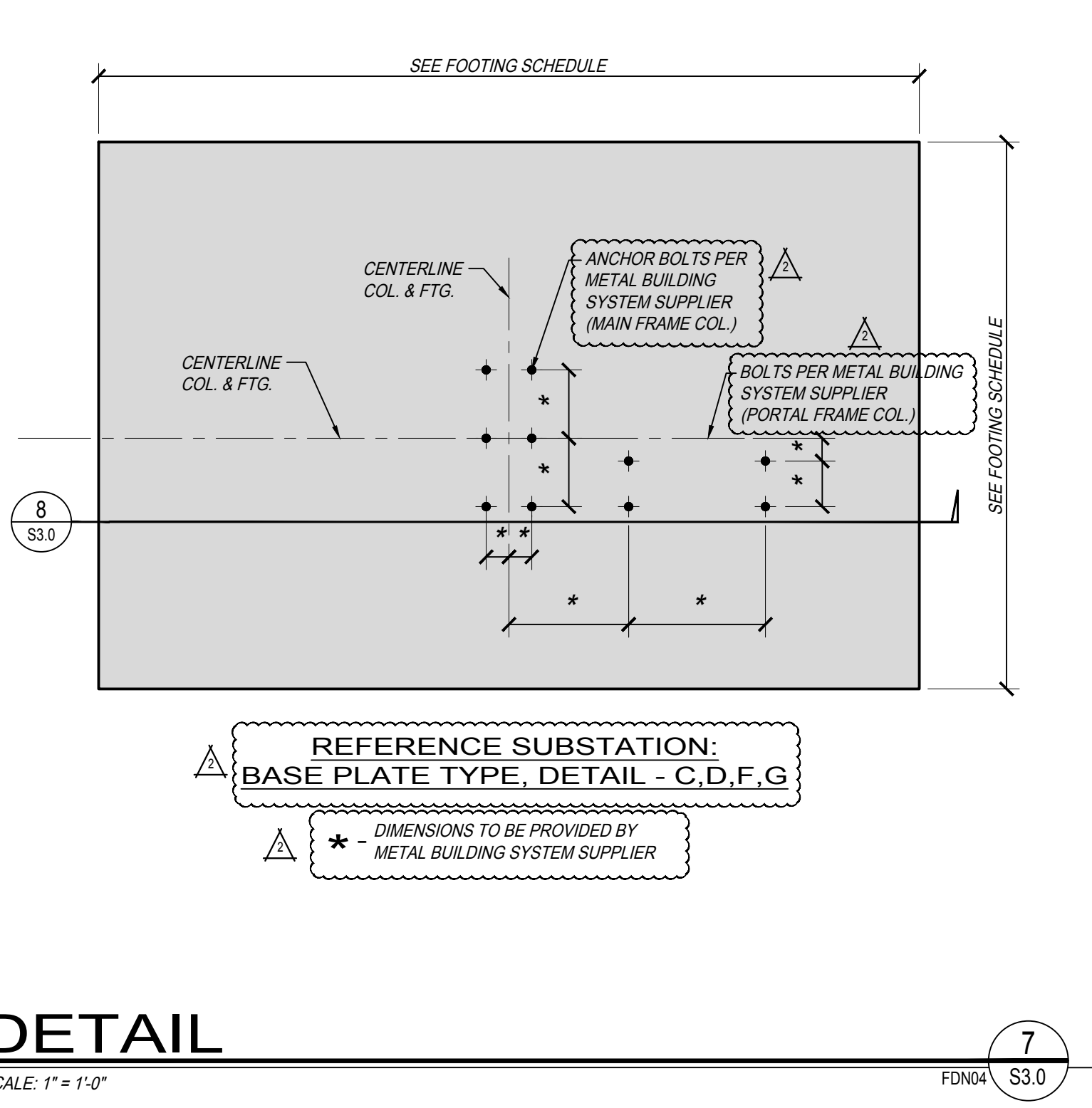
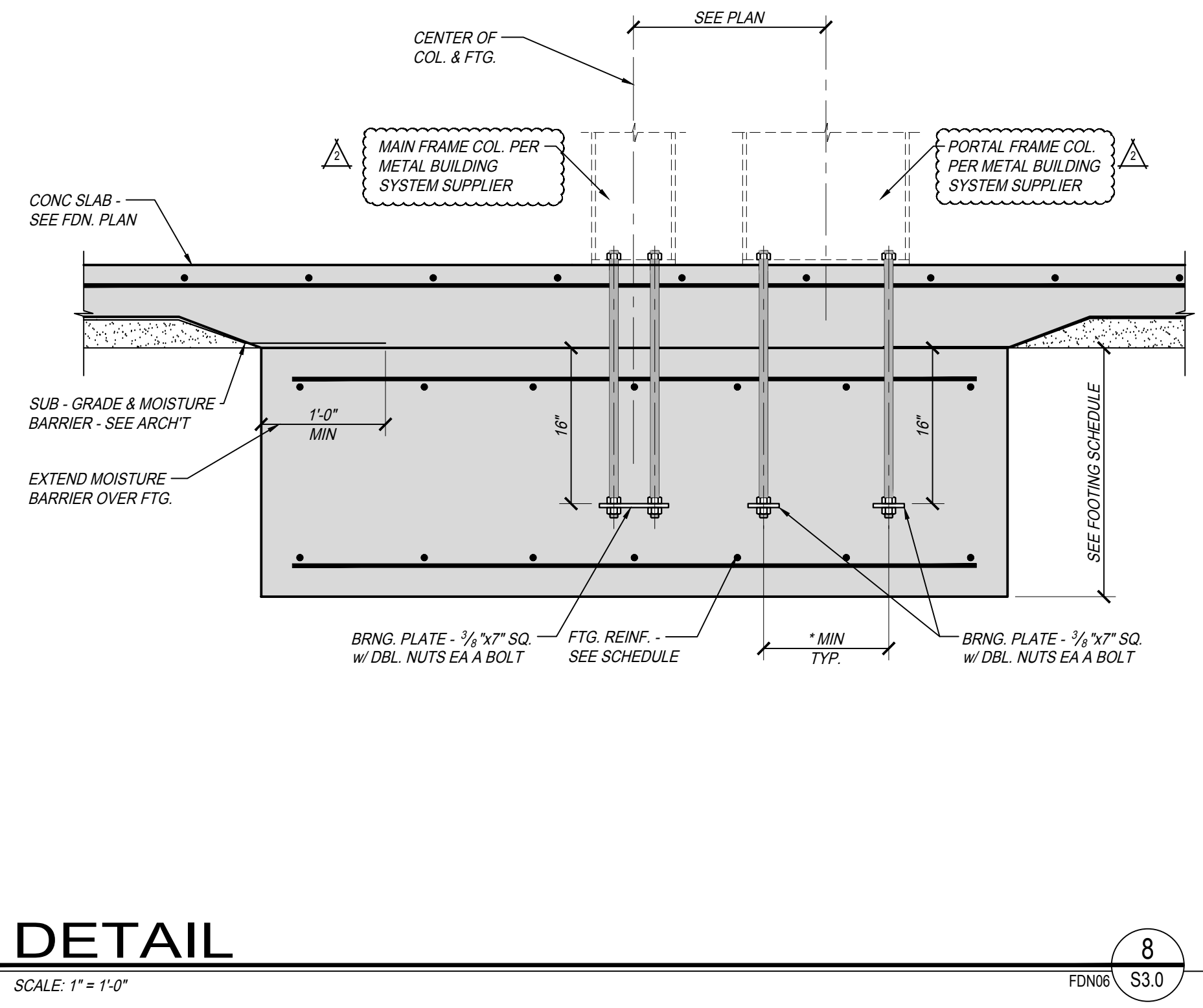
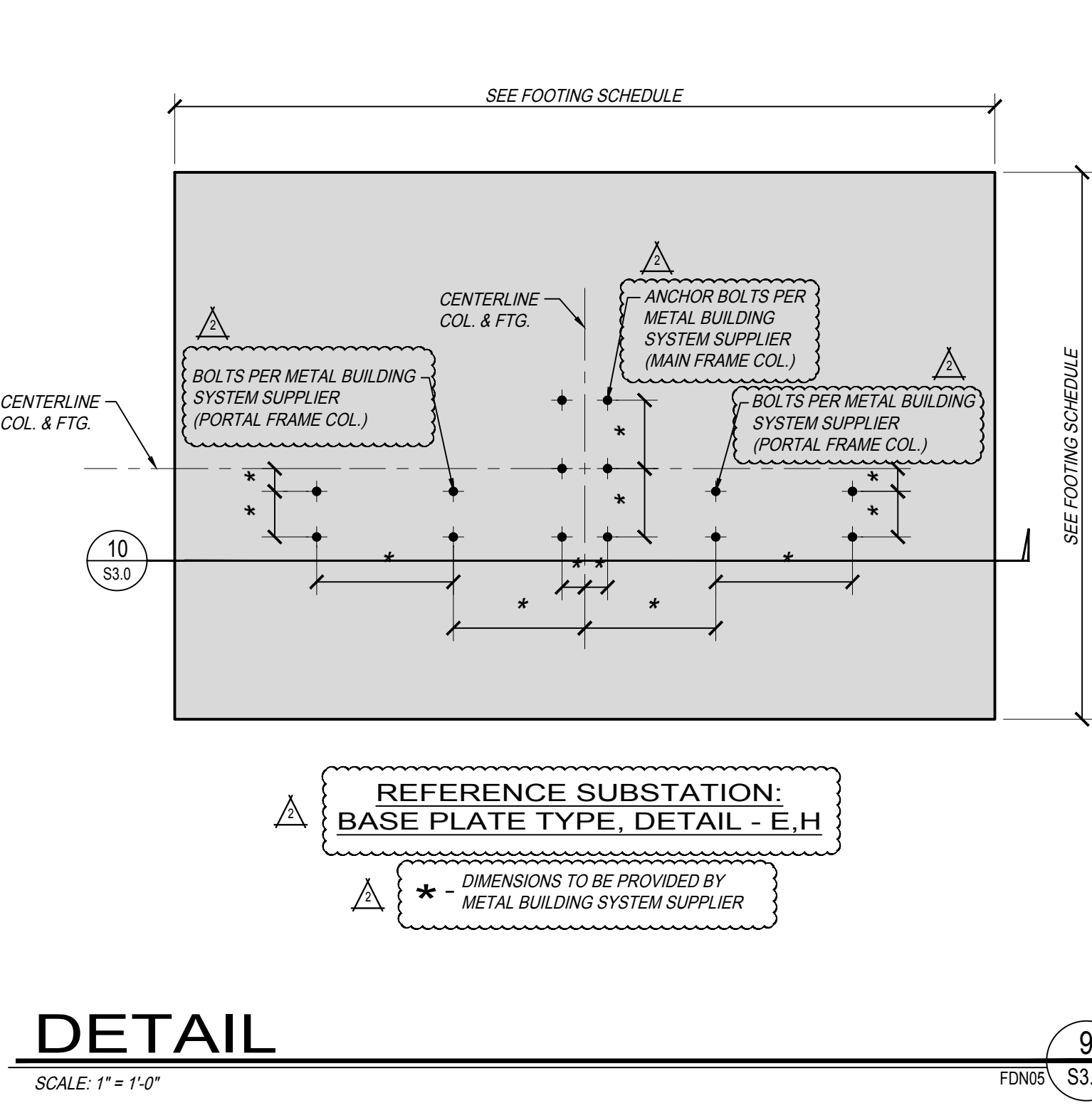
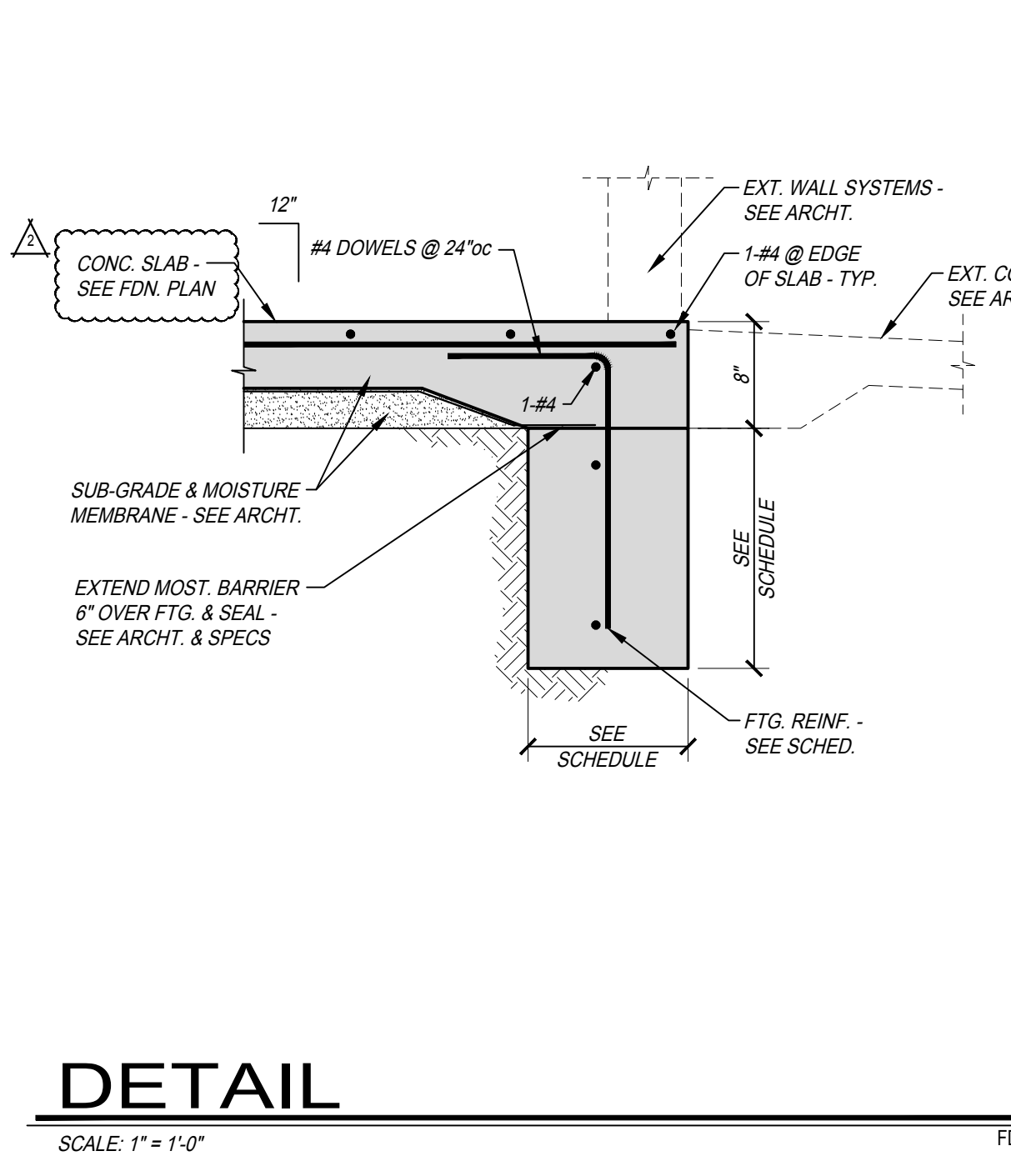
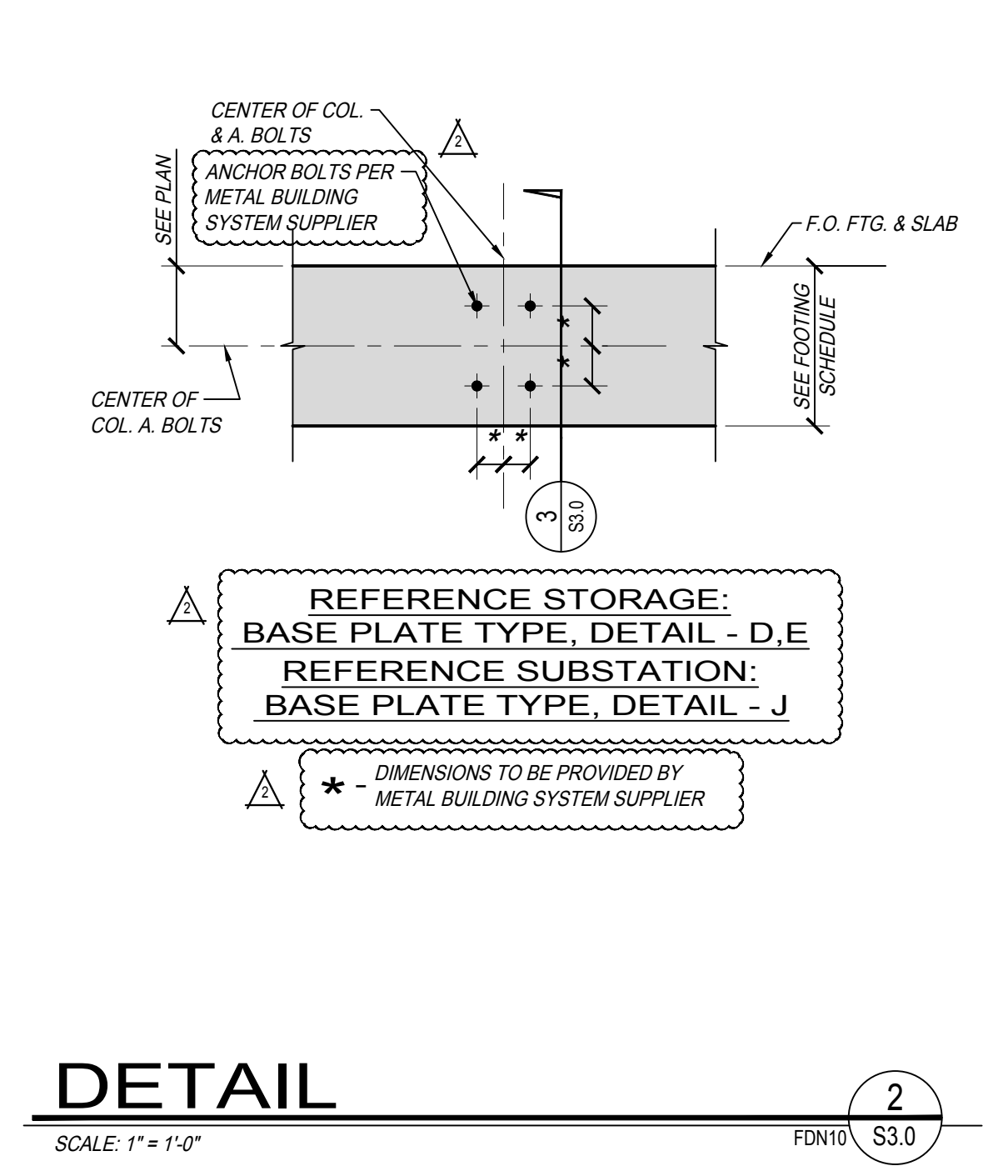
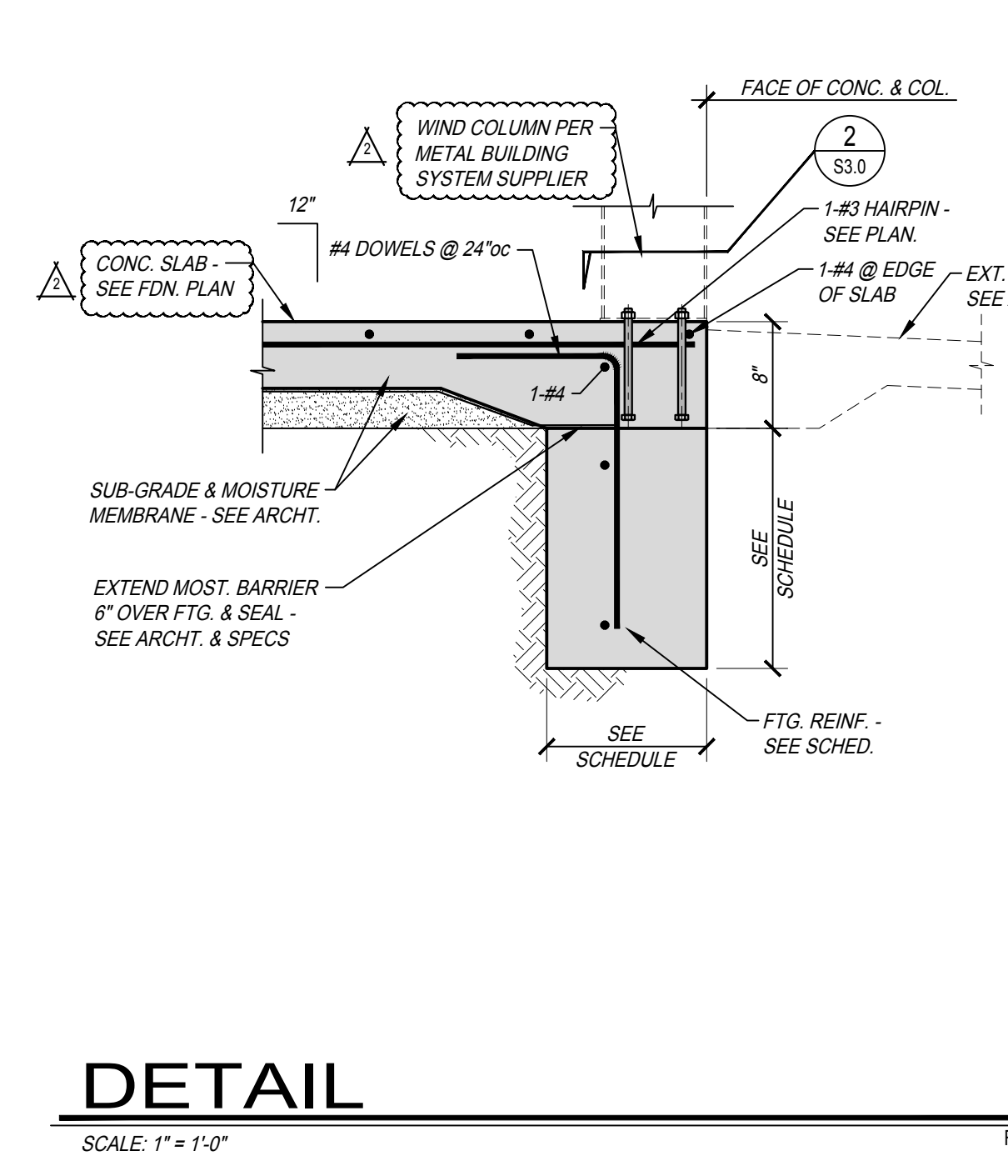
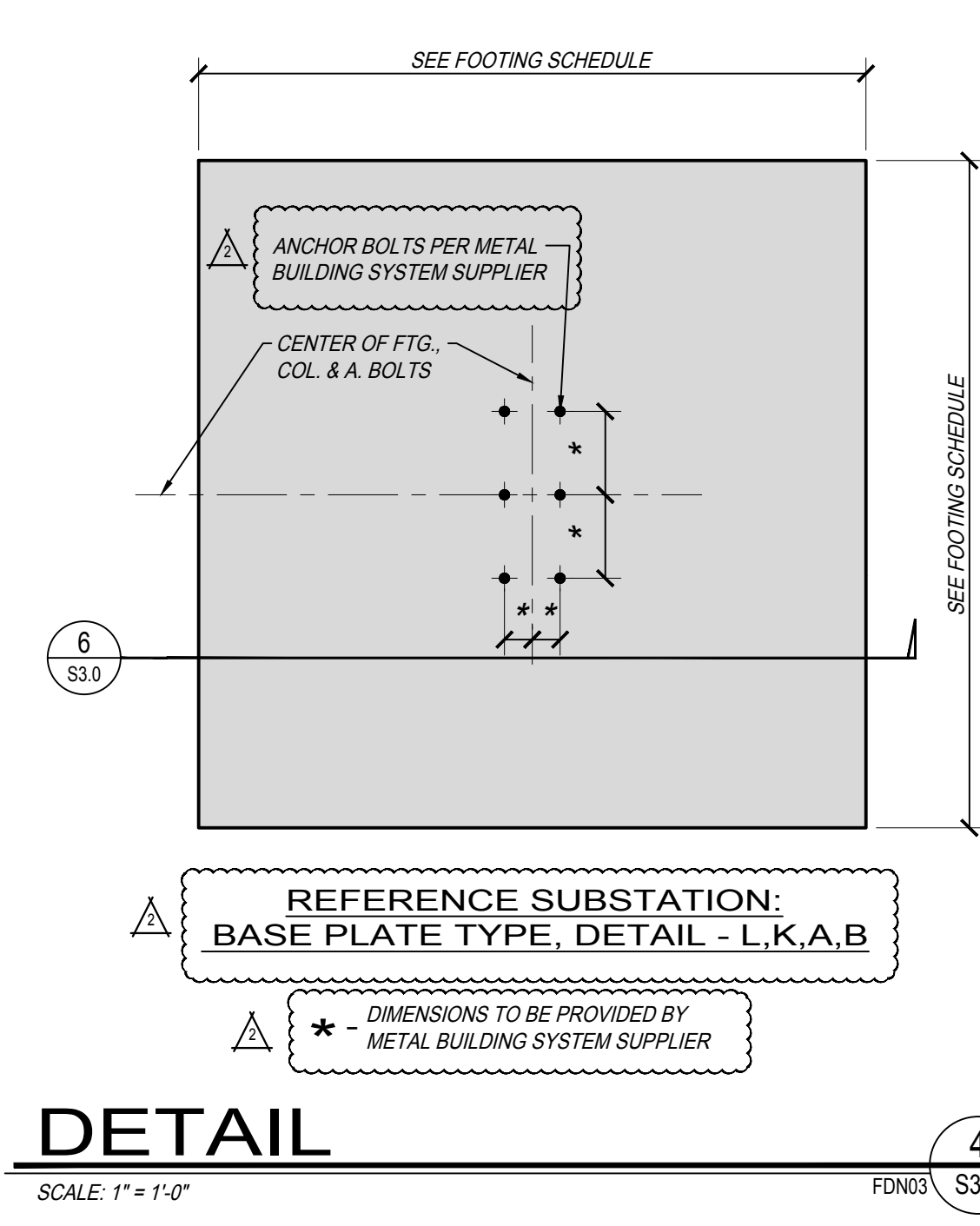
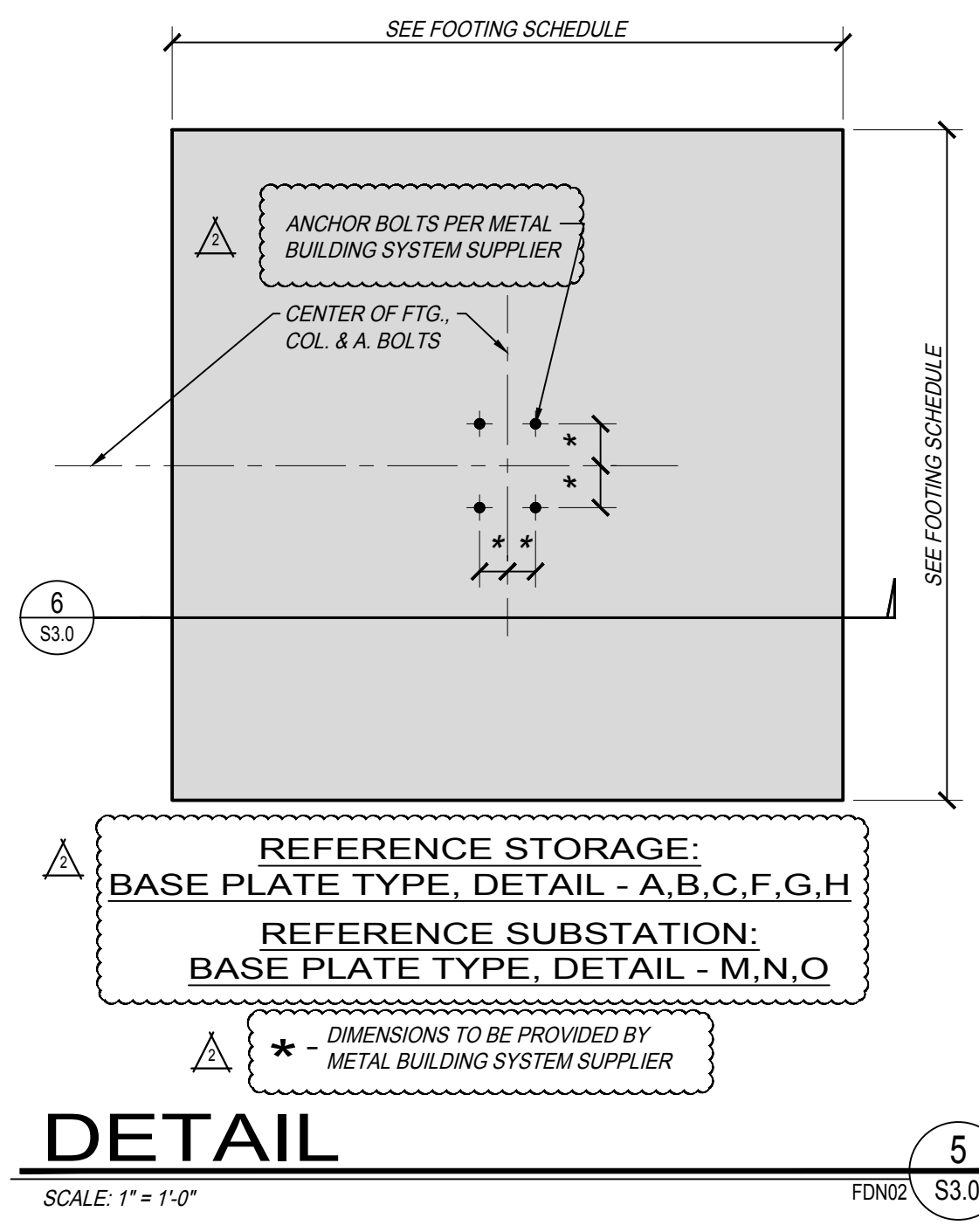
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Capital Projects
2220 Tulare Street, 8th Floor
Fresno, California 93721

Sheet No.
S2.0

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6-16-2020



Project:
Sheriff Area 2 Sub-Station
1129 N. Armstrong Ave., Fresno, CA
APN: 310-133-04, 05, and 06
ISSUE DATE: 06.17.2020
PROJECT NO.: 180293 / 19003
FILE NAME: S3.0

Sheet Content:
FOUNDATION DETAILS

Fresno County Department of Public Works and Planning Capital Projects
2220 Tulare Street, 8th Floor
Fresno, California 93721

Sheet No.

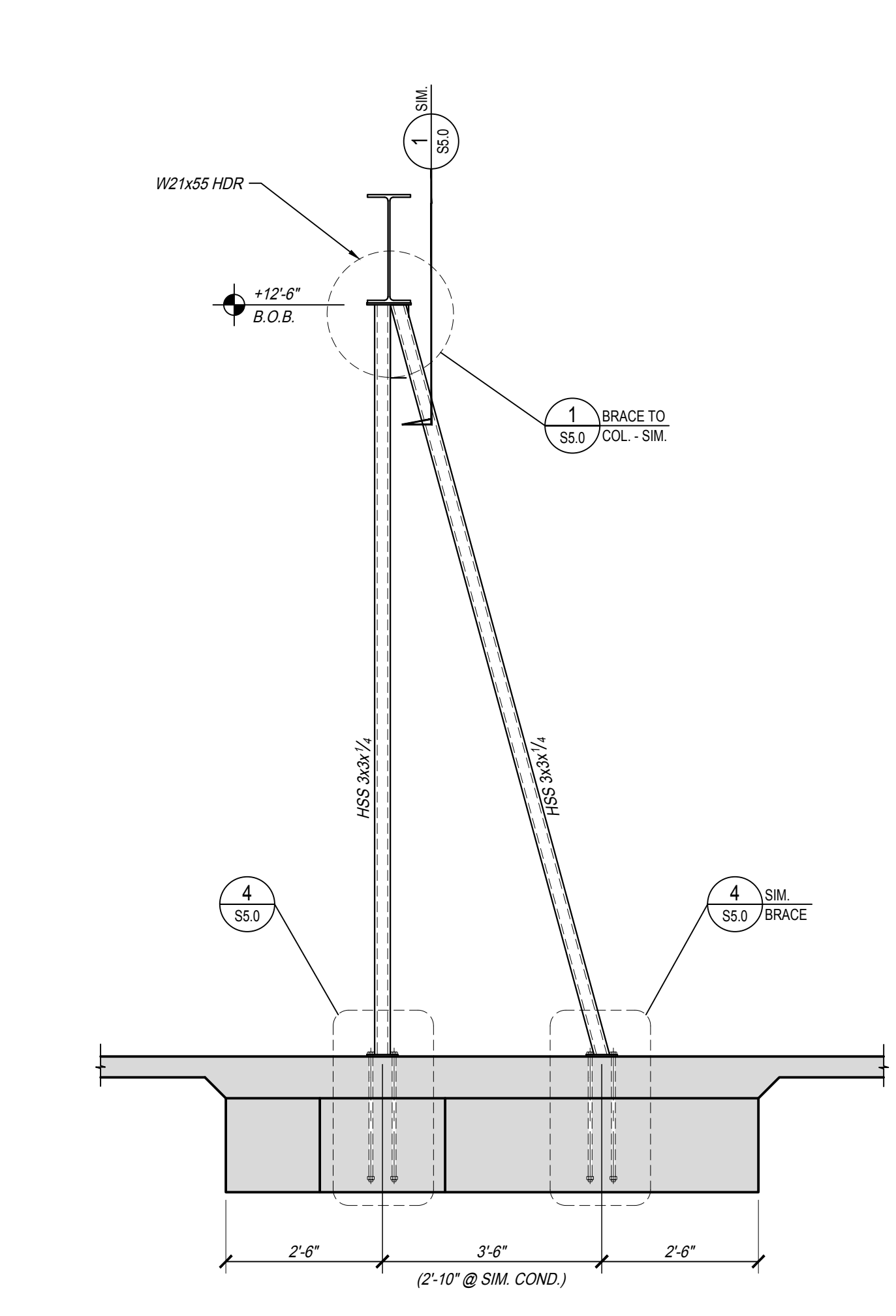
S3.0

Drawn by: SMP Plot date: 06.17.2020

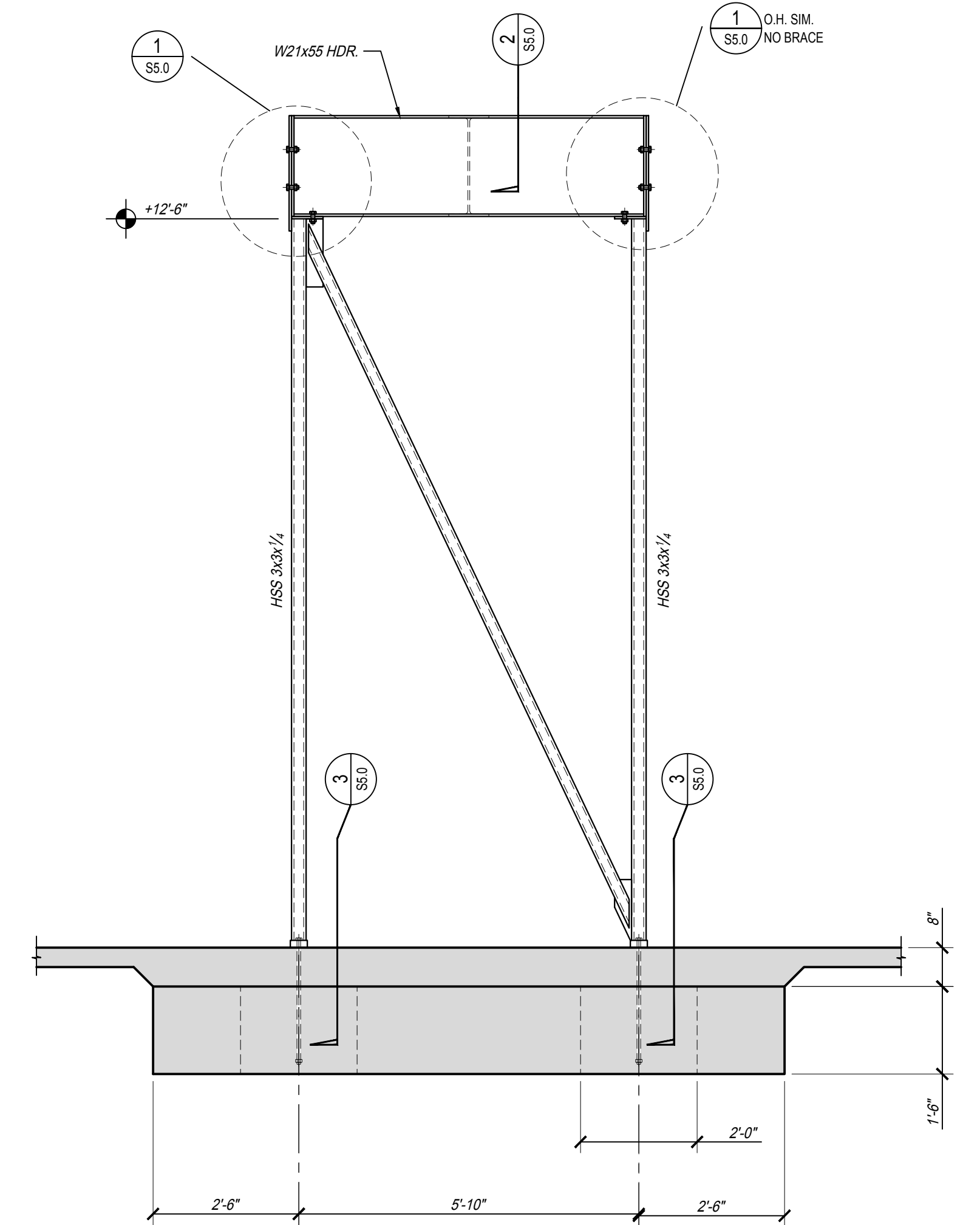


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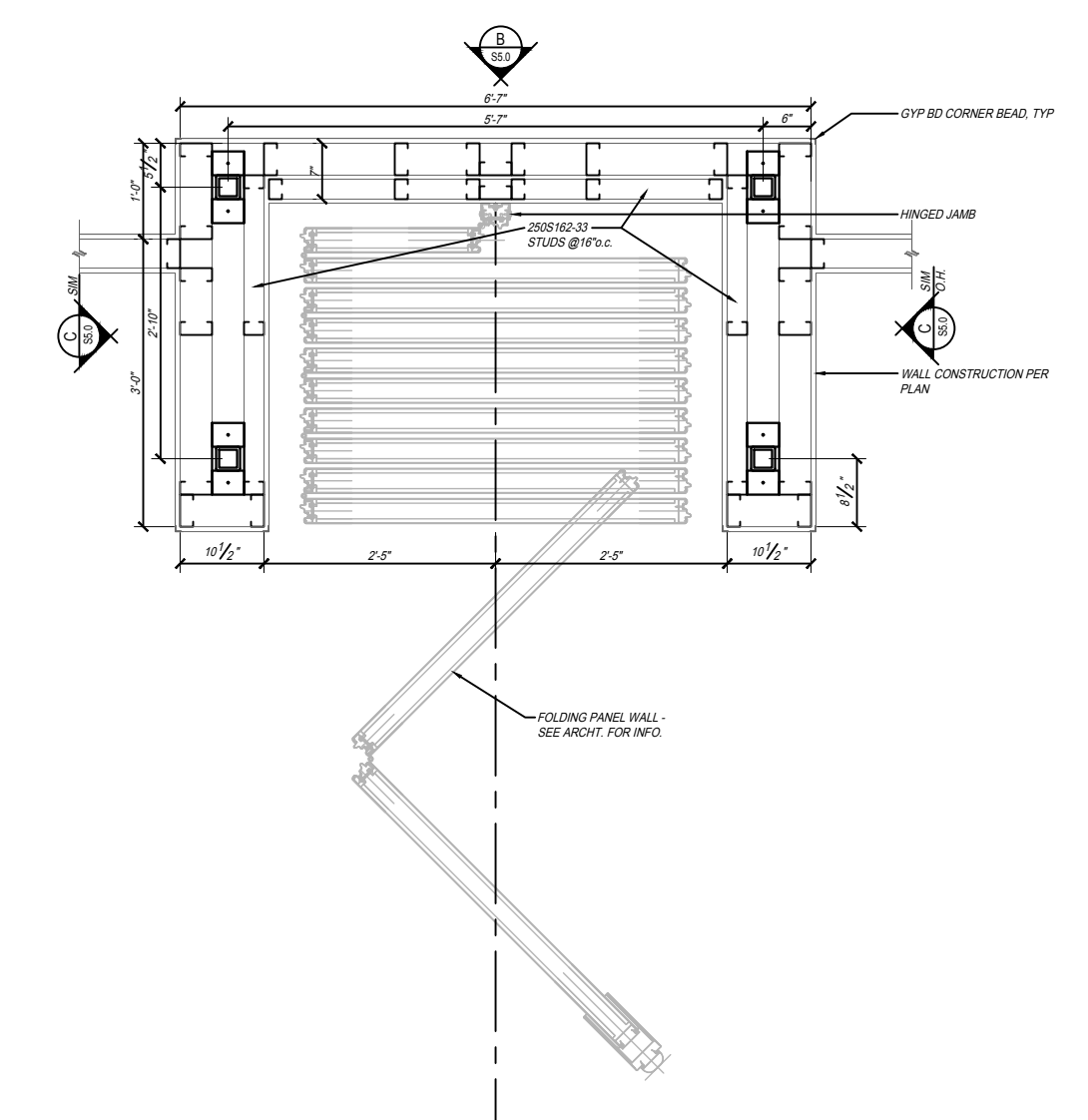
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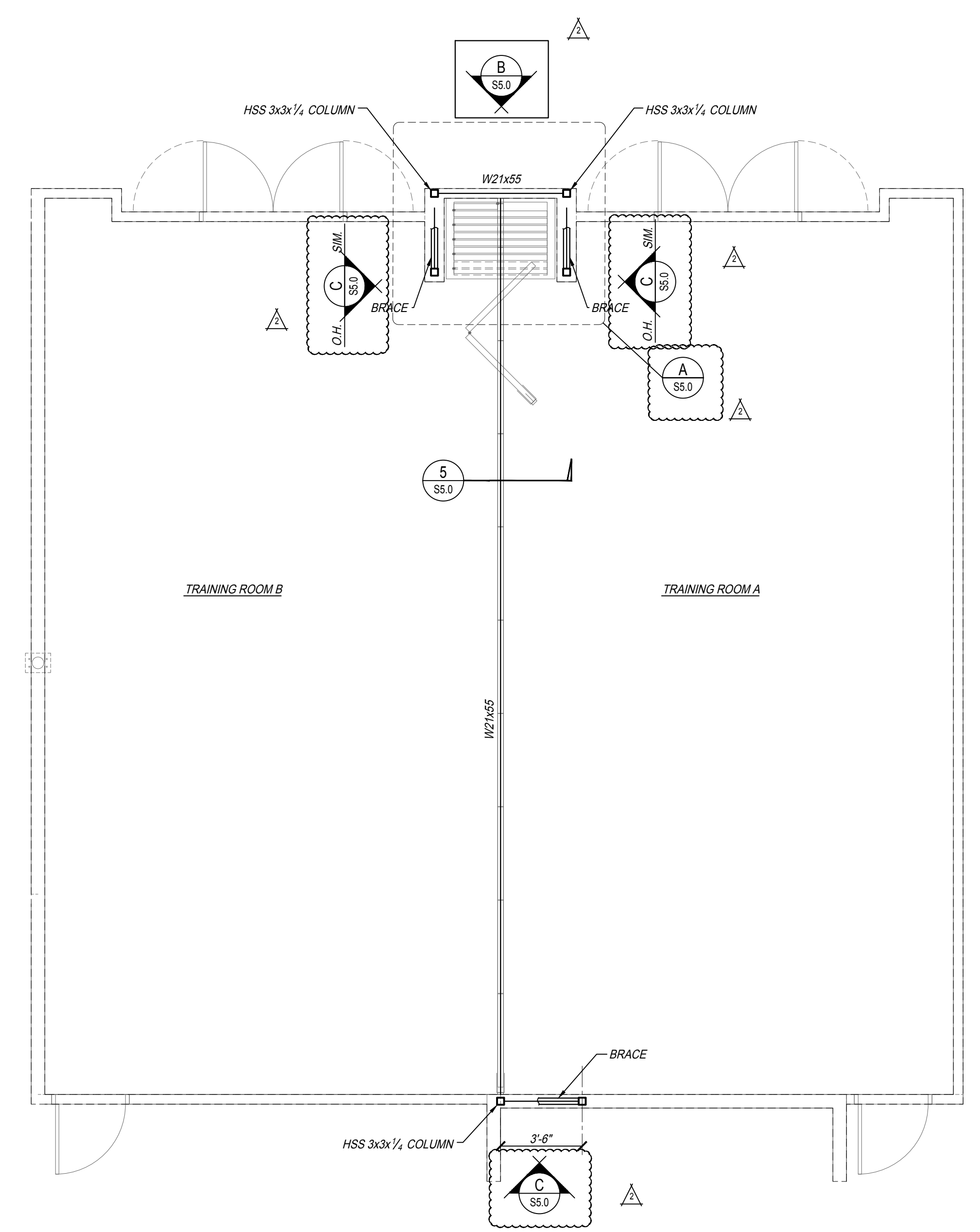
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SECC03 S5.0



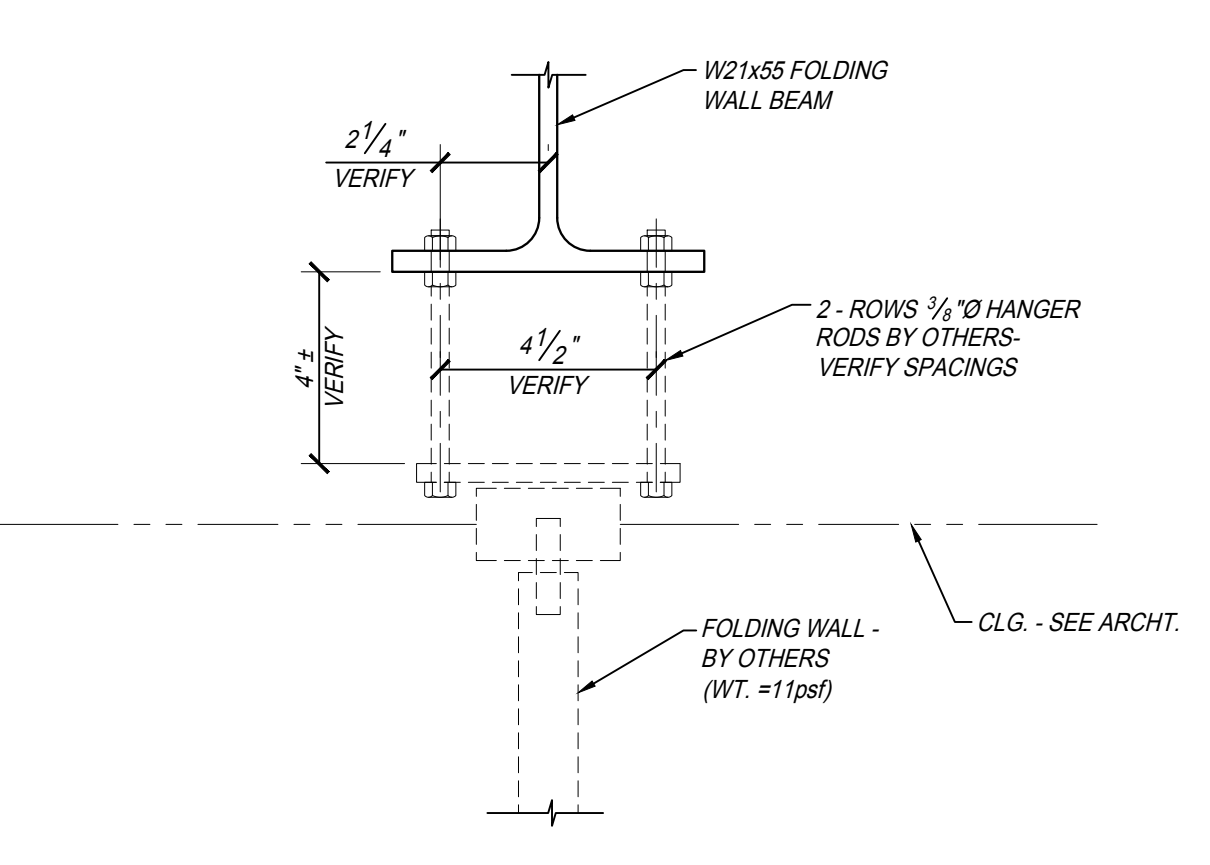
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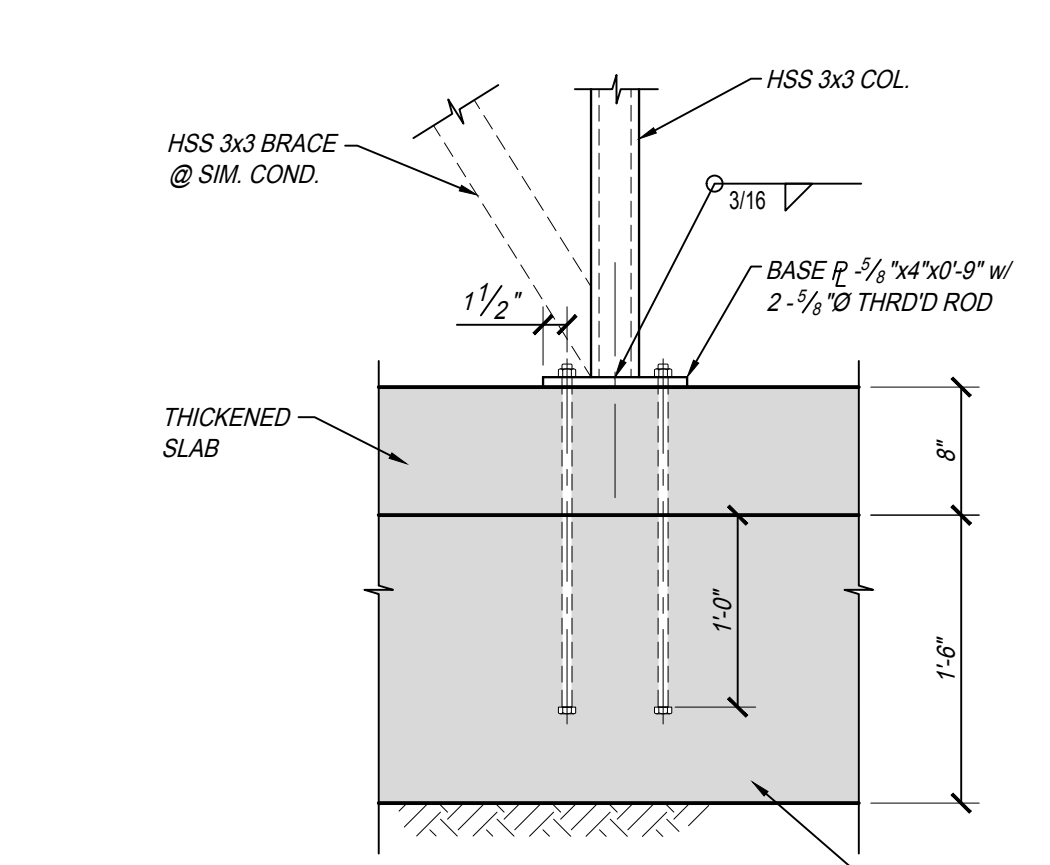
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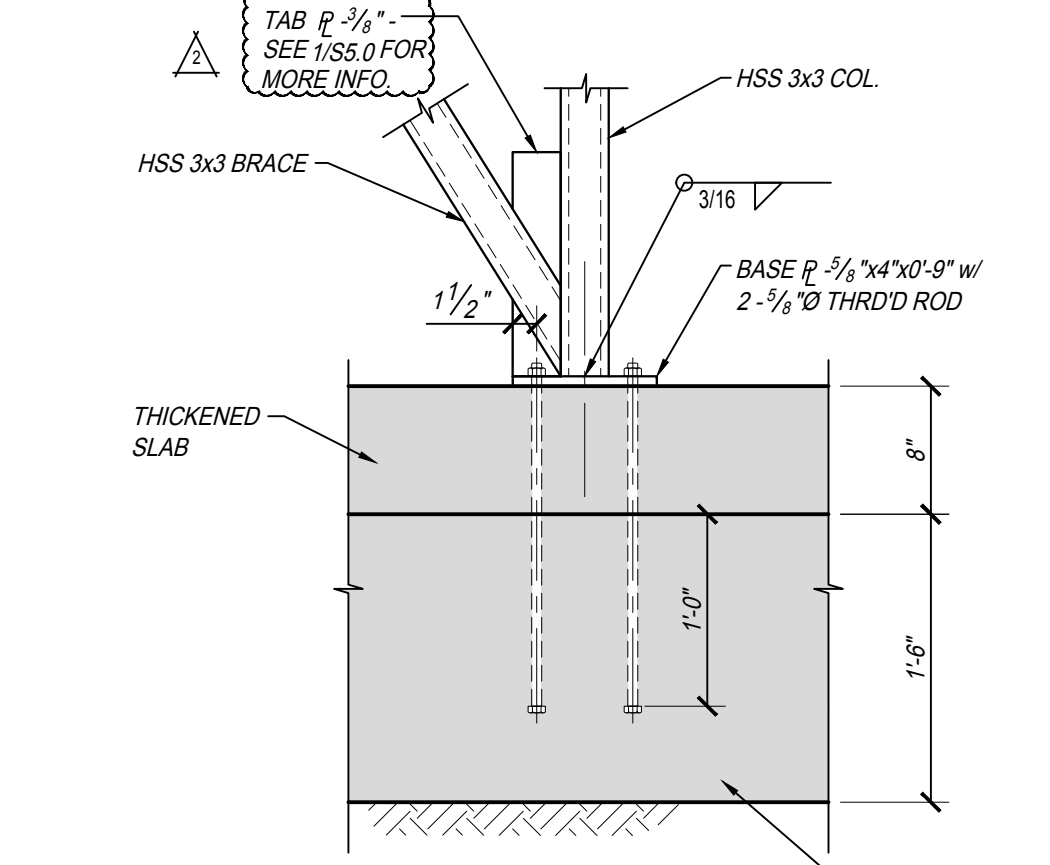
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SCALE: 1/4" = 1'-0"



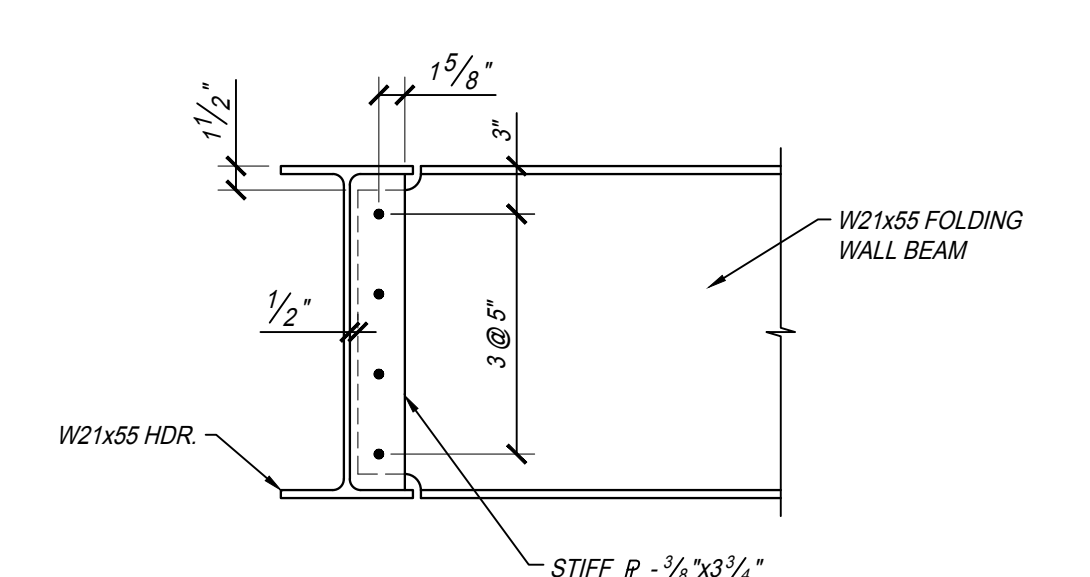
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DETO05 S5.0



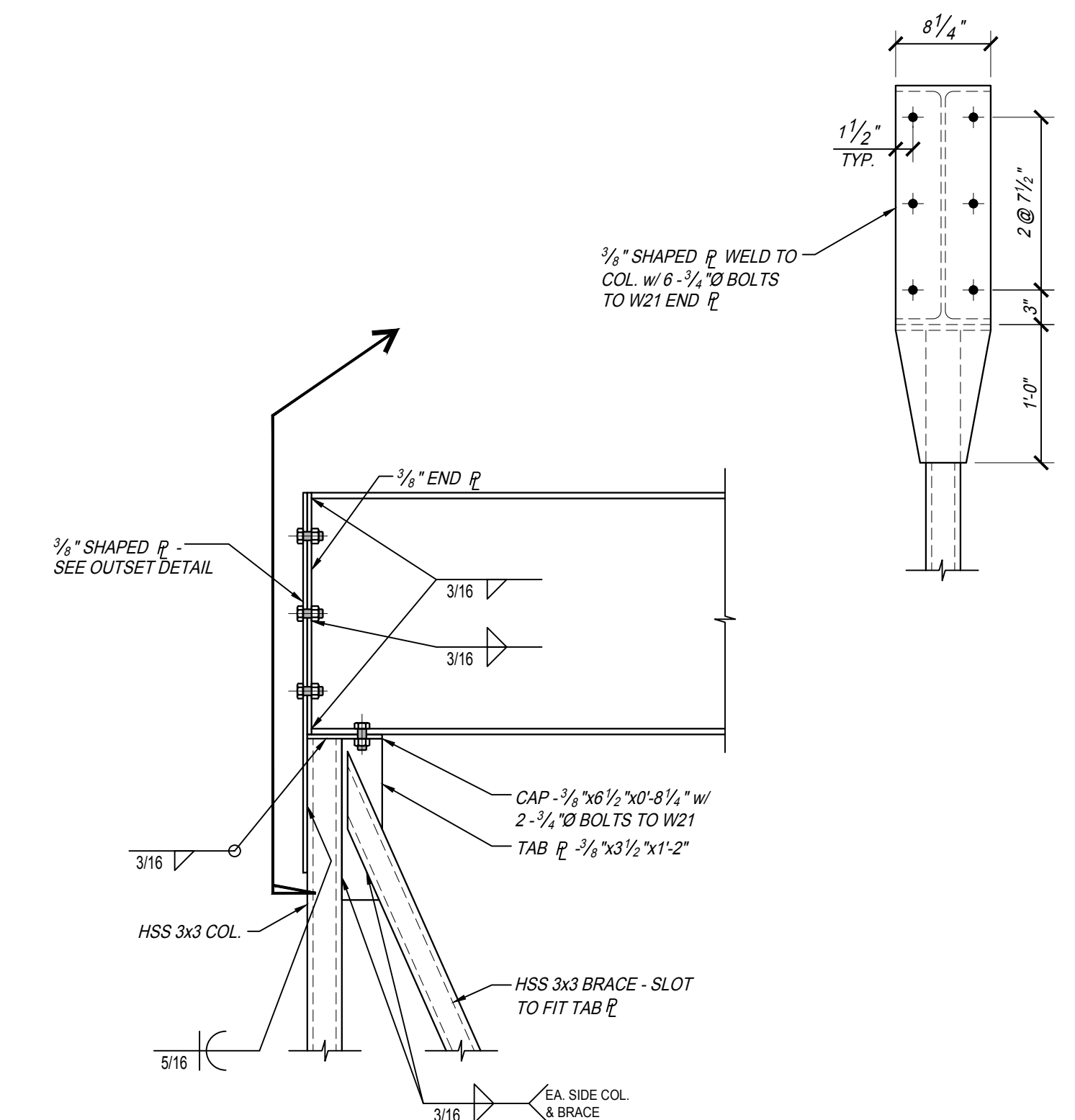
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DETO04 S5.0



DETAIL 3
SCALE: 1" = 1'-0"
DETO03 S5.0



DETAIL 2
SCALE: 1" = 1'-0"
DETO02 S5.0



DETAIL 1
SCALE: 1" = 1'-0"
DETO01 S5.0

6-16-2020

REGISTERED ARCHITECT
Noel Roger Davidson, A.I.A., Architect
California Licensed Architect No. C-27818
Rtn. 10-31-2019
Fresno County Department of Public Works
Capital Projects
2220 Tulare Street, Eighth Floor
Fresno, California 93721
Telephone: (559) 600-4477
E-mail: ndavidson@co.fresno.ca.us

Project:
Sheriff Area 2 Sub-Station
1129 N. Armstrong Ave., Fresno, CA
APN: 310-133-04, -05, and -06
ISSUE DATE: 06.17.2020
PROJECT NO.: 180293 / 19003
FILE NAME: S5.0

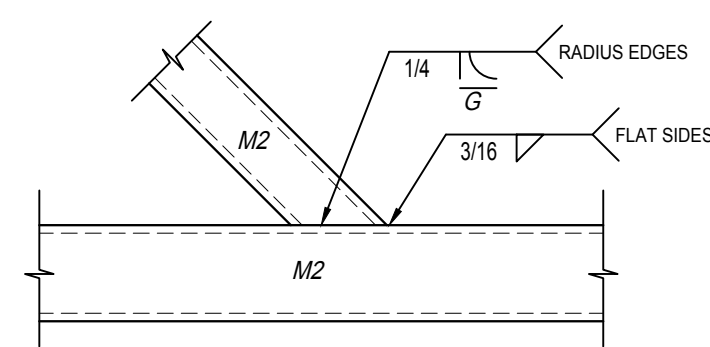
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Fresno County Department of Public Works and Planning
Capital Projects
2220 Tulare Street, 8th Floor
Fresno, California 93721

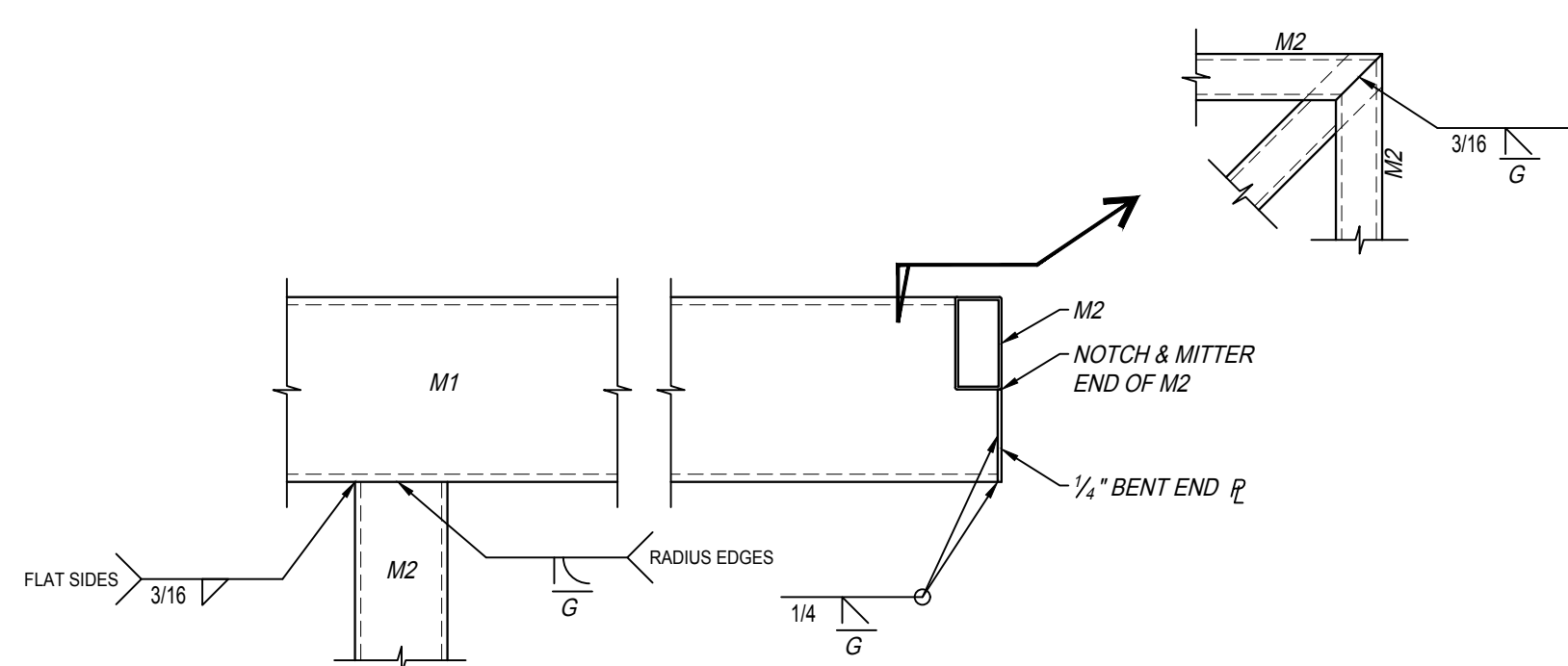
Sheet No.
S5.0

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STRUCTURAL ENGINEERS
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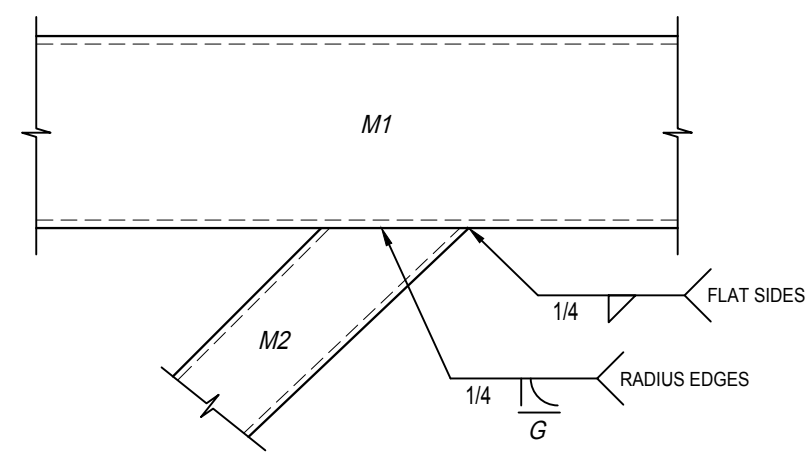
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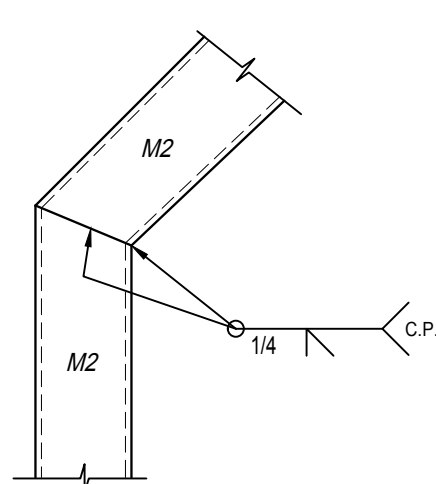
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DET05 S6.0



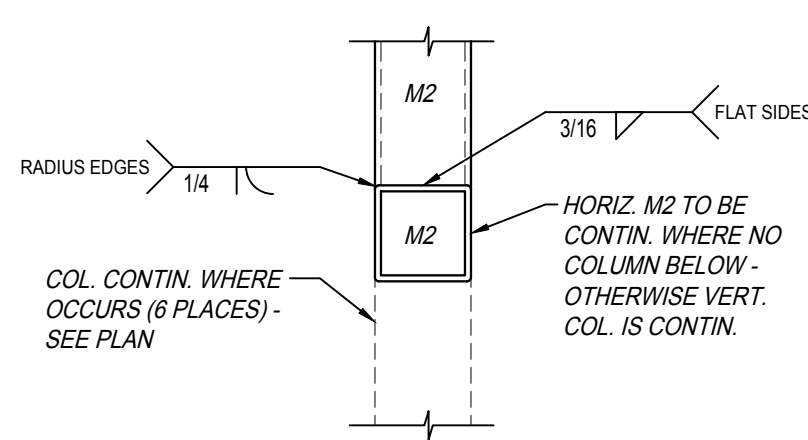
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DET04 S6.0



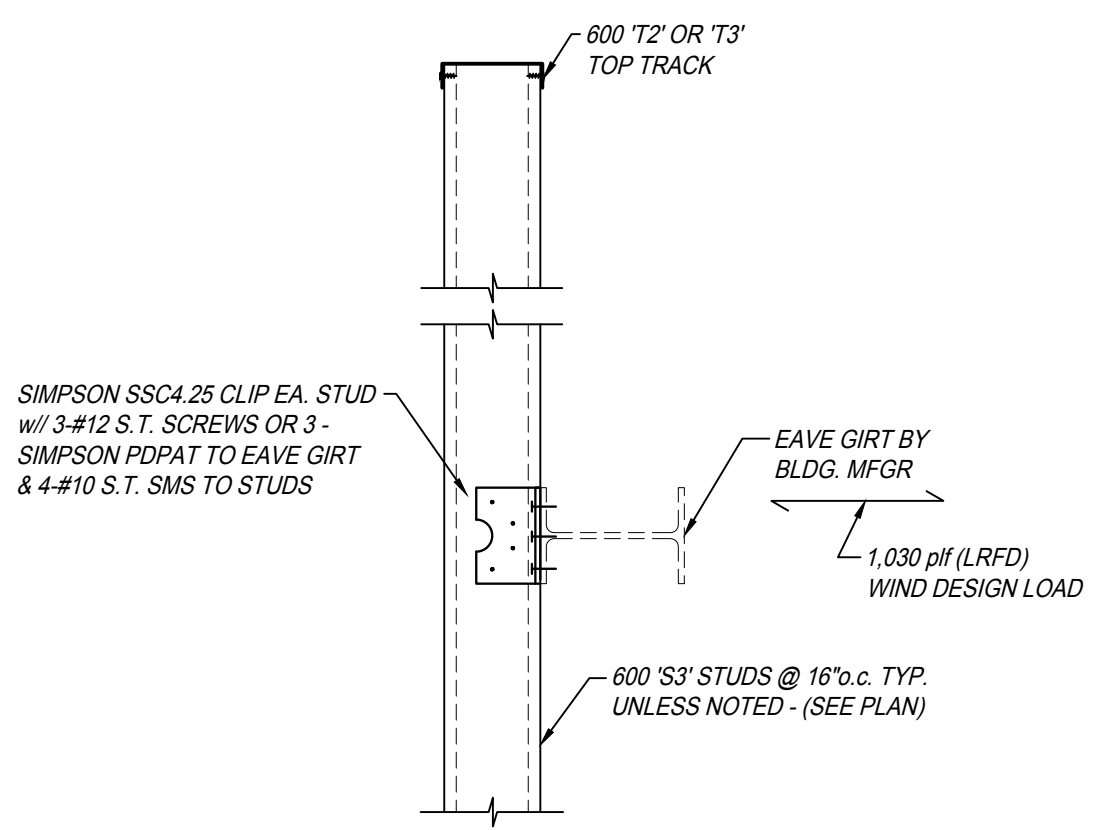
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DET03 S6.0



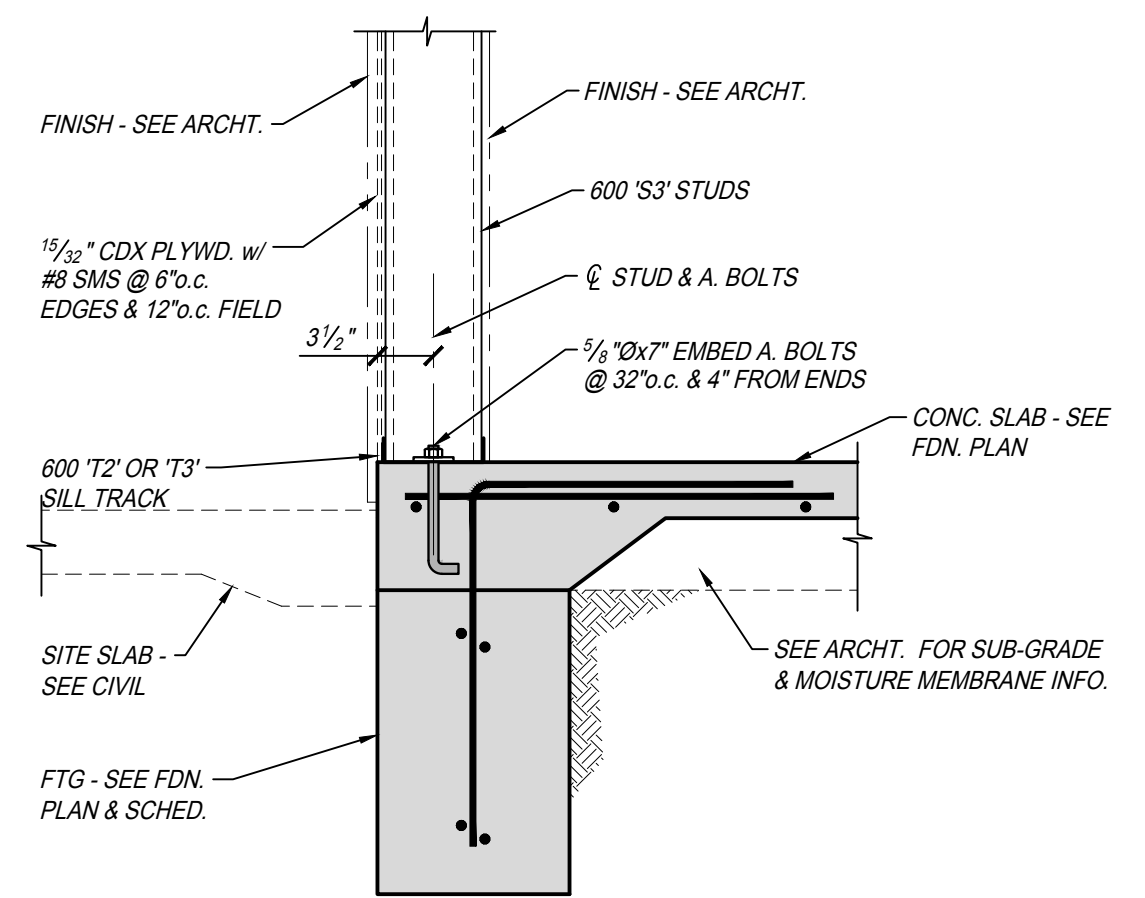
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DET02 S6.0



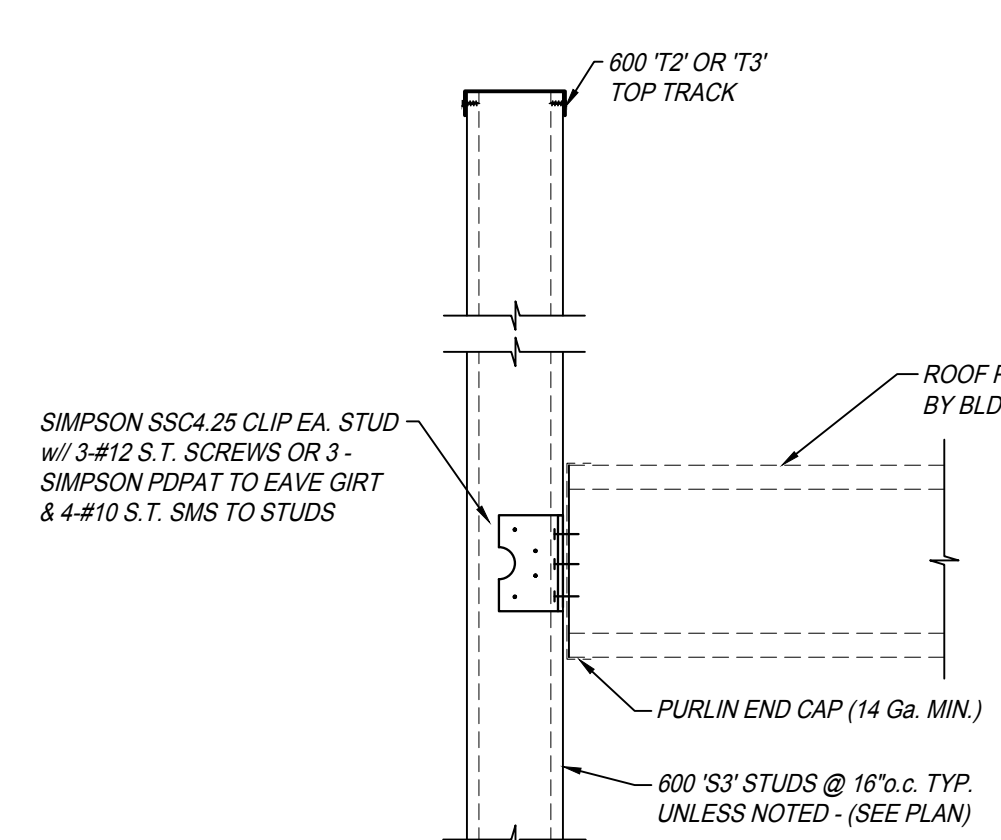
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DET01 S6.0



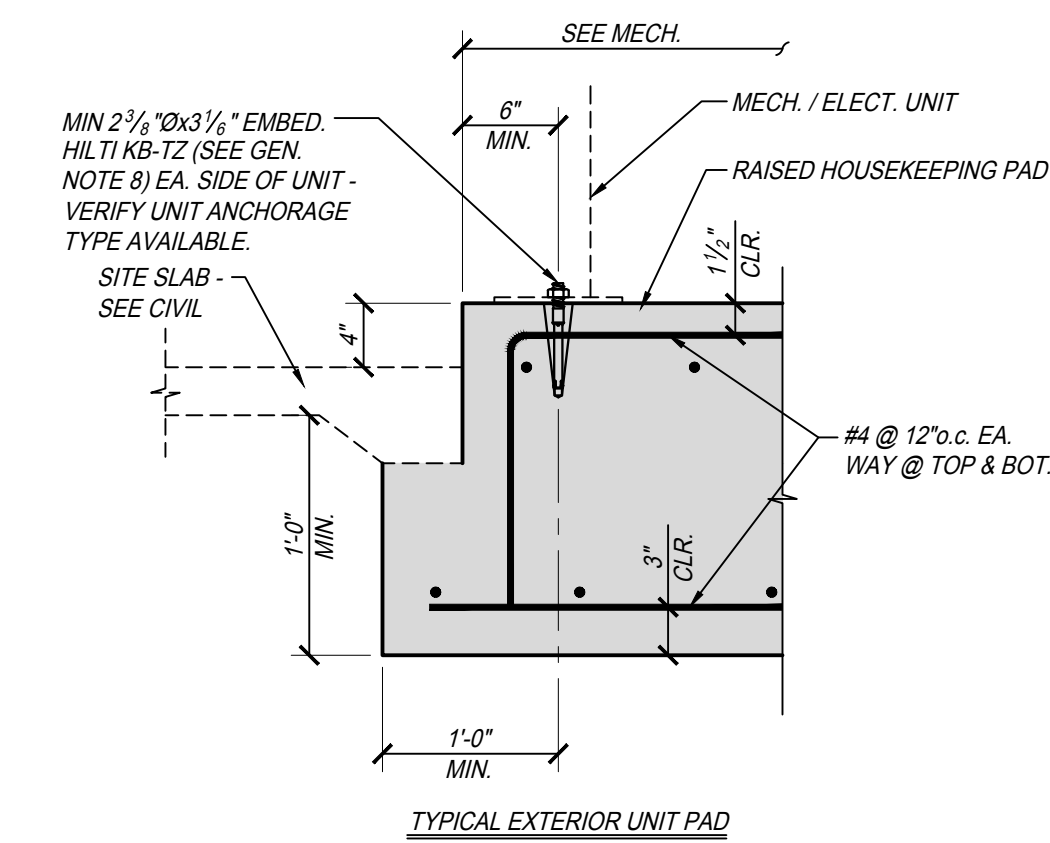
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DET10 S6.0



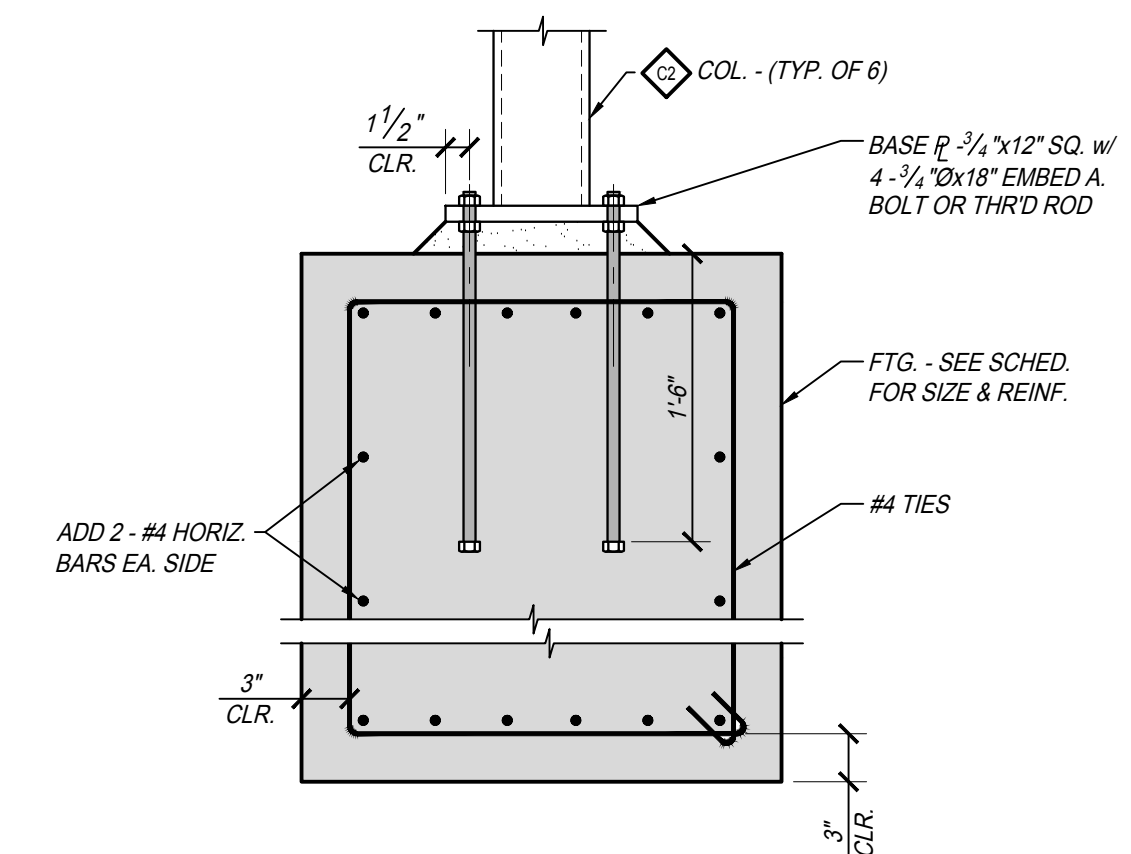
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DET09 S6.0



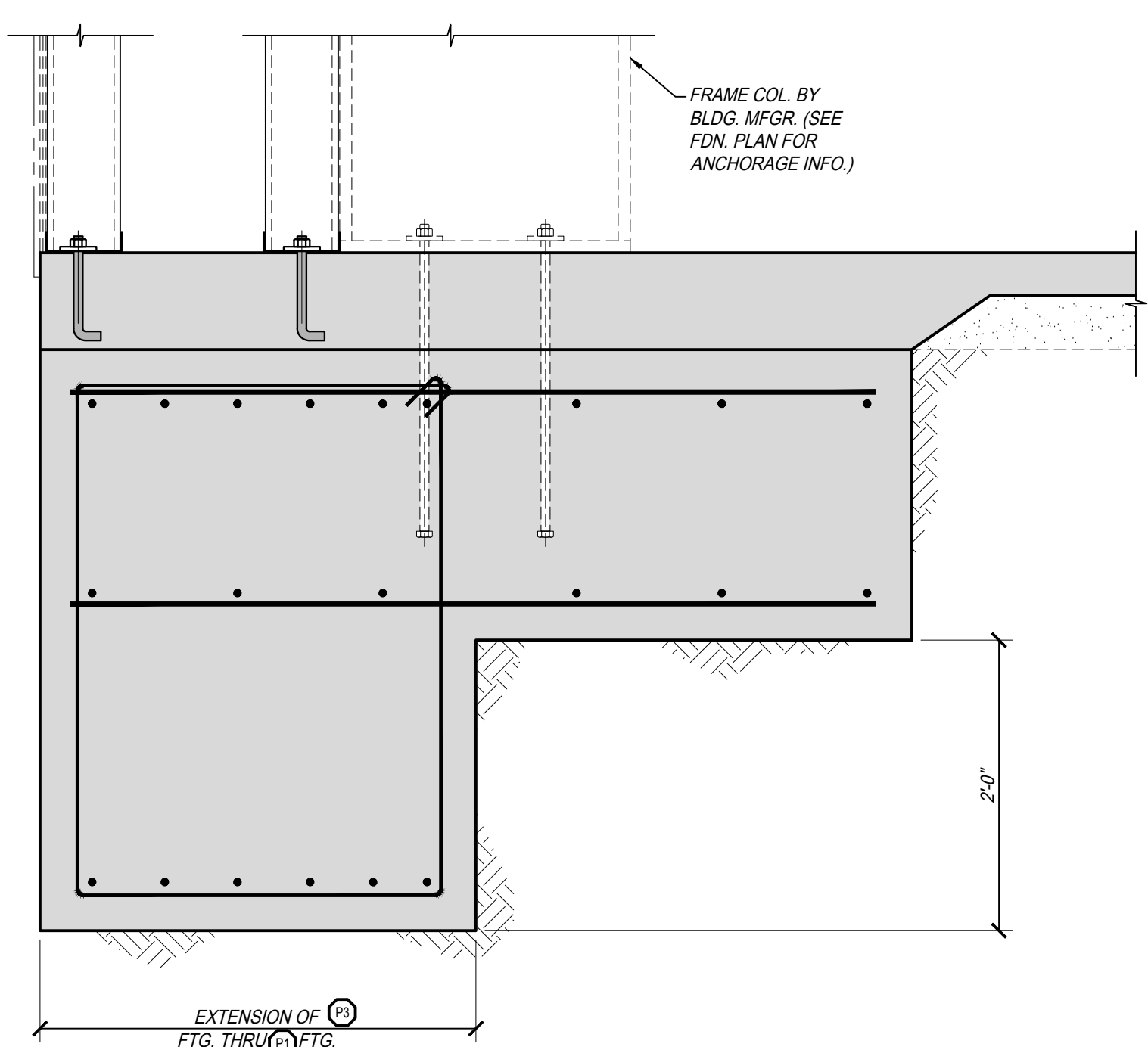
DETAIL 8
SCALE: 1" = 1'-0"
DET08 S6.0



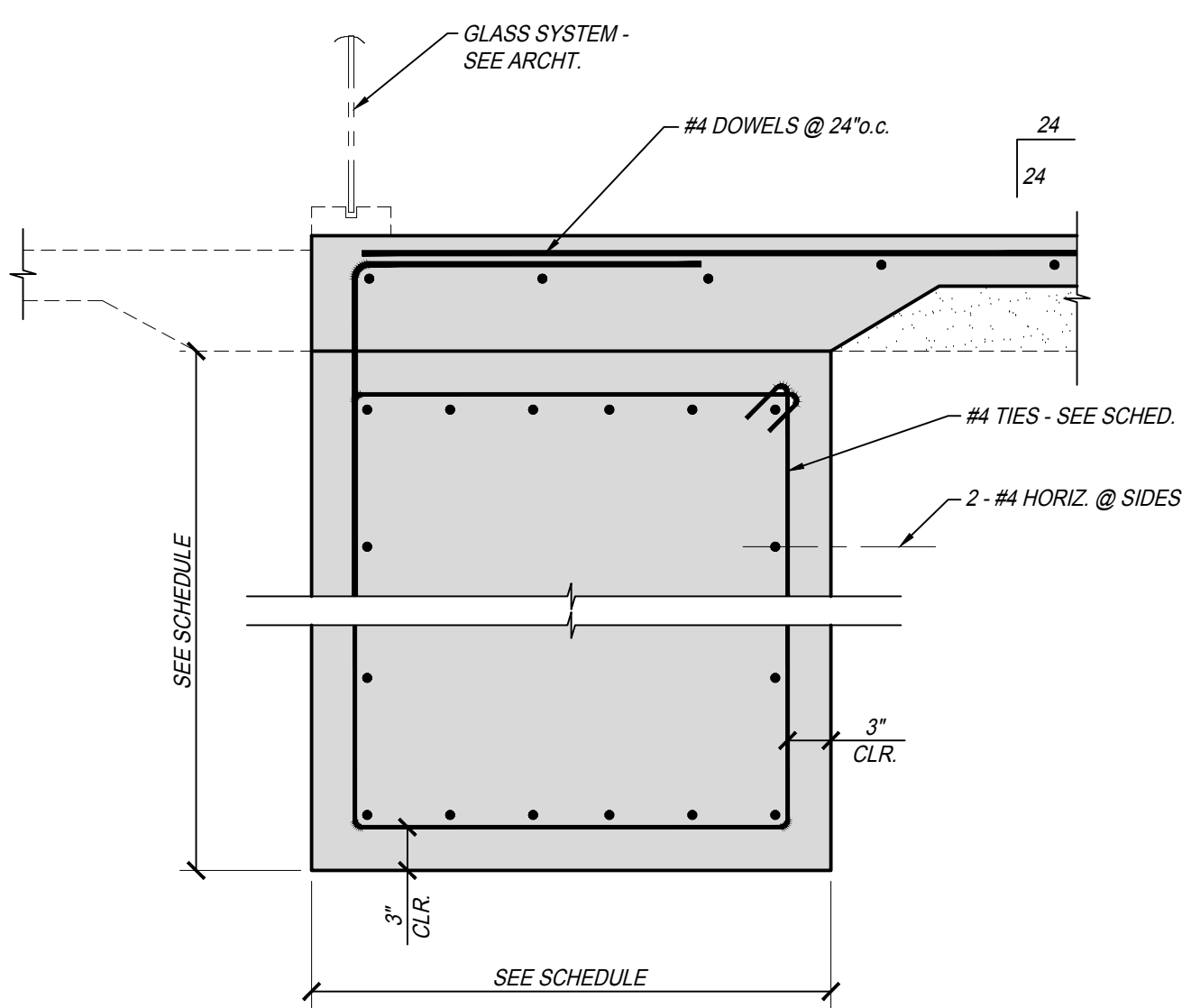
DETAIL 7
SCALE: 1" = 1'-0"
DET07 S6.0



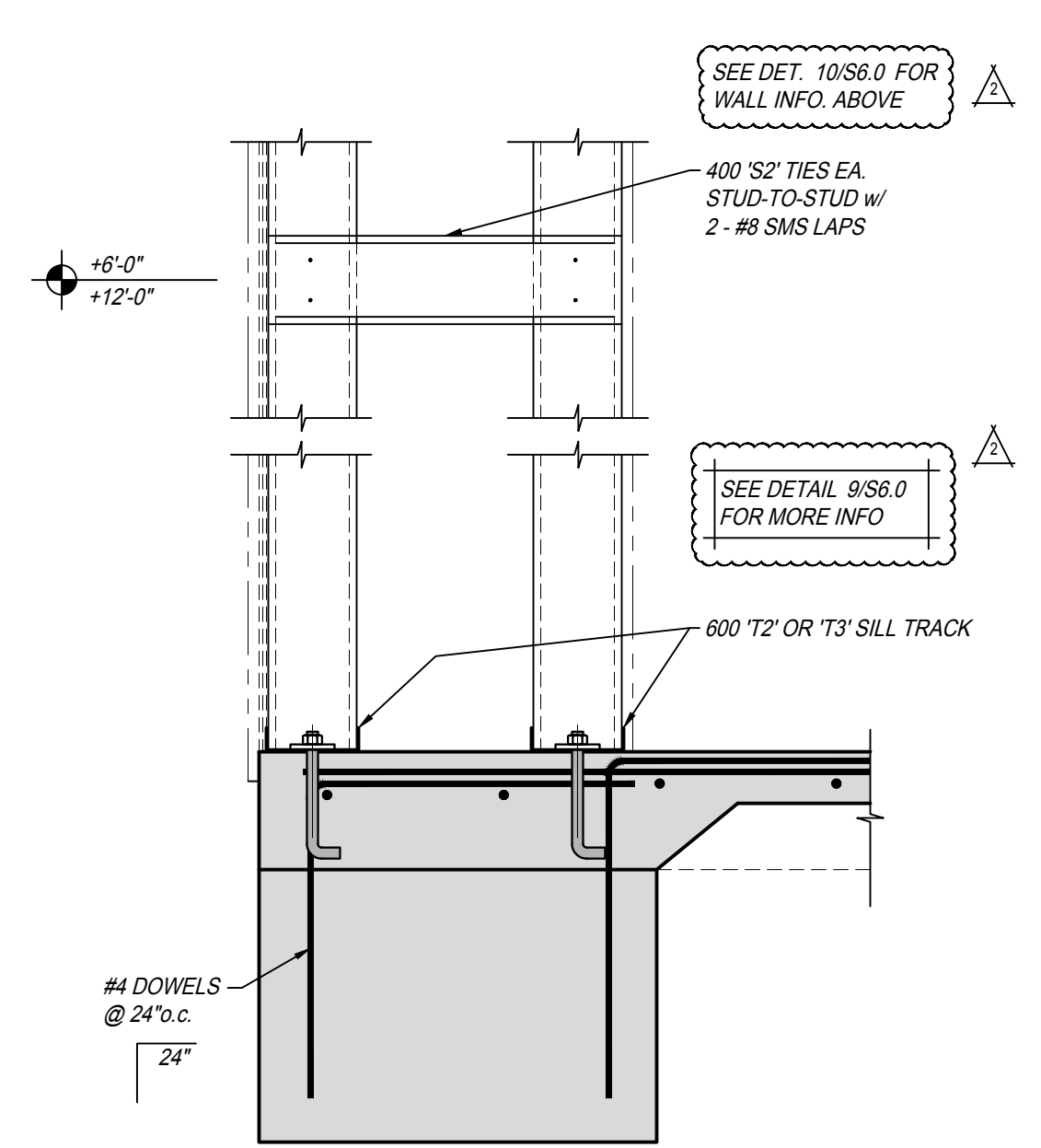
DETAIL 6
SCALE: 1" = 1'-0"
DET11 S6.0



DETAIL 13
SCALE: 1" = 1'-0"
DET13 S6.0

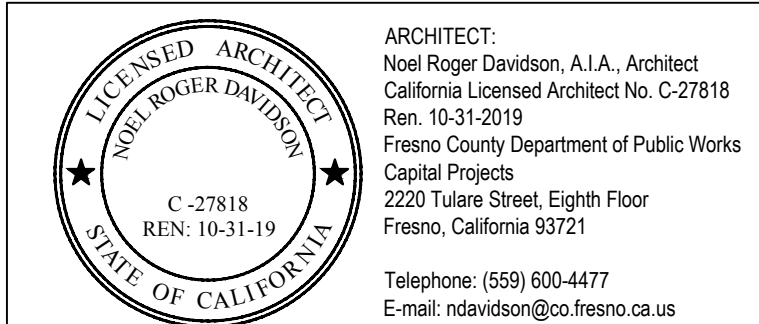


DETAIL 12
SCALE: 1" = 1'-0"
DET12 S6.0



DETAIL 11
SCALE: 1" = 1'-0"
DET11 S6.0

6-16-2020



Project:
Sheriff Area 2 Sub-Station
1129 N. Armstrong Ave., Fresno, CA
APN: 310-133-04, -05, and -06
ISSUE DATE: 06.17.2020
PROJECT NO.: 180293 / 19003
FILE NAME: S6.0

Sheet Content:
DETAILS

Fresno County Department of Public Works and Planning
Capital Projects
2220 Tulare Street, 8th Floor
Fresno, California 93721

Sheet No.
S6.0

Drawn by: SMP Plot date: 06.17.2020



THIS DRAWING SHALL NOT BE USED FOR CONSTRUCTION UNLESS IT BEARS THE STAMPS AND SIGNATURES OF THE ARCHITECT AND ENGINEER AND THE APPROVAL STAMP OF THE JURISDICTIONAL BUILDING DEPARTMENT.